# BEFOREIAFTER SAFETY ANALYSES 

## Prepared for:

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## INTRODUCTION

The purpose of this study was to determine the effects of safety improvements on safety performance at locations chosen by the Colorado Department of Transportation (CDOT). This report discusses the results at 12 locations that were analyzed and the methodology used in the process. In addition, this report discusses the need to institutionalize the process of evaluating safety outcomes of constructed projects.

An overview of the methodology used in the before/after analysis for each location is provided in Appendix A.

## ANALYSIS AND RESULTS

Fifteen locations were chosen by CDOT for analysis for this study. Those locations included state highways and non-state highways covering a variety of safety improvements. Analyzed roadway improvements included: guard rail, cable rail, concrete median, a weather warning system, and deer fencing. Intersection improvements analyzed included: a new signal, additional turn lanes, improving geometry to get rid of split phases, adding protected left-turn phasing, and signal upgrades such as larger signal heads and replacing old span-wire signals.

Three of the non-state highway study locations had poor data availability, and we were unable to accurately analyze them without implementing unified street naming convention and manual quality control for the off system crashes. The remaining 12 projects were analyzed and are provided in Table 1 with the location, type of project, and resulting benefit/cost (B/C) ratio. As shown, many of the B/C ratios were greater than anticipated at the time of application for funding. Of the 12 safety projects analyzed, 3 showed no improvement or deterioration in safety performance in the after period and may not have been justified. The 3 projects with little to no improvement included:

- \#15505 - Deer fencing and cattle guards on US 550. The number of wild animal crashes was reduced following construction as would be expected. However, the crashes were more severe in the after period causing the B/C ratio to be below one.
- \#16006 - Intersection improvements at SH 45 and Red Creek Springs. The number of crashes in the before and after period were approximately the same, but the severity of crashes increased in the after period. It is unclear why the severity of crashes increased following this improvement project.
- \#16010 - New signal at Industrial and Purcell. The number of broadside crashes decreased after the signal was constructed, but several other crash types saw an increase in number of crashes including approach turns, rear-ends, and sideswipes. In addition, the severity of crashes increased. The signal was warranted, but the results suggest that an intersection with volumes that just meet warrants might have better safety outcomes with a roundabout.

Table 1. Summary of Safety Analyses Locations

| Def. | Region | Highway/Intersection | MP | Improvement Type | Initial Predicted B/C | $\qquad$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| \#15505 | 5 | US 550 | 107.00-111.00 | Deer Fencing/Cattle Guards | 1.81 | 0.24 |
| \#15645 | 6 | SH 83 | 69.39-70.57 | Concrete Median | 2.11 | 5.91 |
| \#15748 | 6 | 1-76 | 1.77-5.78 | Cable Rail | 14.36 | 6.16 |
| \#15770 | 2 | I-25 | 102.50-107.50 | Median Cable Rail | 2.65 | 2.26 |
| \#15771 | 2 | SH 165 | 18.65-23.90 | Guard Rail | 4.97 | 12.67 |
|  |  | SH 93/SH 72 | 7.57 | Signal Upgrade | 1.72 | 14.93 |
| \#15828 | 6 | SH 93 | 7.47-11.83 | Weather Related Road Closure System | 1.17 | 1.42 |
| \#15861 | 4 | SH 52 / 95th Street | 3.16 | Intersection Improvements | 2.52 | 13.37 |
| \#15862 | 4 | US 34 / 11th Avenue | 112.23 | Intersection Improvements | 2.03 | 9.69 |
| \#15900 | 3 | SH 133 | 46.00-51.50 | Guard Rail | 4.89 | 21.54 |
| \#16005 | 2 | US 50 / Purcell Boulevard | 309.78 | Intersection Improvements | 1.77 | 4.00 |
| \#16006 | 2 | SH 45 / Red Creek Springs | 3.95 | Intersection Improvements | 1.18 | 0.08 |
| \#16010 | 2 | Industrial / Purcell | - | New Signal | 1.12 | - |

Appendix B provides a detailed report for each study location providing all the analyses and results.

All the barrier improvements including guard rail, cable rail, and concrete barrier showed significant improvement with B/C ratios ranging from 2.26 to 21.54 , indicating that these are excellent safety improvements when crash data indicates there is a run-off-the-road pattern. The primary goal of these roadway barriers is to reduce the risk of severe crashes that can occur when a vehicle leaves the roadway. The barrier helps to reduce severe crash types such as head-on, overturning, or tree crashes. In the two guard rail studies (\#15771, \#15900), it was found that not only did the severity of crashes decrease, but the number of vehicles leaving the roadway also decreased. It has been theorized that perhaps the guard rail not only acts as a barrier to help reduce severe crashes, but it also provides additional delineation helping to keep drivers aware of curves in the roadway and as a result reduces the frequency of run-off-the-road crashes.

There were several projects that involved improving existing intersections and upgrading signals that had $B / C$ ratios greater than one. These improvements including upgrading old signal heads to 12 inch heads, fixing geometry so split phasing is not required, adding turn lanes, and adding channelization and protected left-turn phasing. The B/C ratios varied from 4.00 to 14.93 for these projects, indicating that these improvements had a significant impact on the safety of the intersections. As mentioned earlier, there were two intersection improvement projects that did not have a $B / C$ ratio greater than one.

The wide range in the resulting $B / C$ ratios following construction of improvements demonstrates that CDOT should continue conducting the observational before/after safety studies for safety projects. It is important to gather data to learn what safety improvements are the most effective so that CDOT can make improved decisions regarding safety projects in the future.

## SUMMARY AND RECOMMENDATIONS

It is important for CDOT to continue to do these Before/After Safety Analyses to understand what safety improvements are most effective. While many of the projects analyzed in the study have shown significant safety benefits, some showed deterioration in safety. It is essential to complete these studies in order to understand the impacts of different improvements types and why sometimes the anticipated safety improvements are not observed following construction. It is recommended that CDOT institutionalize this process and complete a before/after safety analysis for all safety projects constructed. Analyzing safety performance of projects before and after completion will allow CDOT to make better and more informed decisions for future projects.

## APPENDIX A. STATEWIDE METHODOLOGY

Development of Methodology for Evaluating Changes in Safety Performance on Completed Construction Projects

## By

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## Introduction

The intent of this report is to describe a methodology for evaluating safety outcomes of constructed projects. One of the main sources of factual knowledge about the effect of highway and traffic engineering measures is the 'observational Before-After study'. The term observational in this context is used to distinguish between a randomized experiment designed to answer a research question and observing the safety consequences of some treatment that has been constructed for purposes other than answering a research question. Two kinds of evaluation methods is described here; the first will address safety evaluation methodology applied to the individual project and the second one will be used when estimating Crash Modification Factors (CMF) of a specific safety countermeasure applied to a group of sites.

## Methodology to Evaluate Changes in Safety Performance at an Individual Site or Project

The use of this methodology will be illustrated using a specific example describing safety improvement resulting from constructing a median barrier on I-76 (MP 1.77 to MP 5.78). In this case, a Safety Performance Function (SPF) representing Urban 4-Lane Freeway is available.

Step 1
Identify scope and dates/duration of the construction period, in this case median barrier construction 7/9/2007-10/19/2007.

## Step 2

Using Vision Zero Suite (VZS) collect safety performance data and AADT for 3-5 years of the before period, in this case the 5 years of before period used was $1 / 1 / 2002$ 12/31/2006.

## Step 3

Using VZS evaluate safety performance in the before period following correction for the Regression to the Mean (RTM) bias using Empirical Bayes method. RTM phenomenon reflects the tendency for random events, such as vehicle crashes to move toward the average during the course of an experiment or over time. This is addressed effectively by using the Empirical Bayes (EB) method ${ }^{1}$. The EB method for the estimation of safety increases the precision of estimation and corrects for the regression to the mean bias. It

[^0]is based on combining the information contained in accident counts (known crash history) with the information contained in knowing the safety of similar entities. The information about safety of similar entities is brought into the EB procedure by the SPF through use of expected mean value and over-dispersion parameter associated with the specific SPF. Correcting for the RTM is a default setting in VZS. Figure 1 shows safety performance of I-76 (MP 1.77 to MP 5.78) from the severity standpoint in the before period 1/1/200212/31/2006 EB corrected for RTM.


Figure 1 EB Corrected SPF Inj+Fat - I-76 (MP 1.77 to MP 5.78)
(Before Period - 1/1/2002-12/31/2006)
Step 4
Evaluate safety performance of I-76 (MP 1.77 to MP5.78) [1/1/2008-12/31/2012] in the after period. According to Hauer${ }^{2}$, the crash count in the after period is not subject to the EB correction for the RTM bias. Figure 2 shows how to turn off EB correction in the VZS and Figure 3 shows safety performance in the after period without the EB correction (4.49 crash $/ \mathrm{mi} /$ year) and the before period corrected for RTM ( $6.23 \mathrm{crash} / \mathrm{mi} /$ year) on the same graph.

[^1]

## Figure 2 EB Correction Turned Off



Figure 3 SPF Inj+Fat - I-76 (MP 1.77 to MP 5.78)
(EB Corrected Before Period- 1/1/2002-12/31/2006) and (After Period - 1/1/2008-12/31/2012)
Step 5
Establish what the safety of the site in the after period would have been had safety improvement not been constructed and compare it with the after period. This is accomplished by first computing the percentile of the EB corrected safety performance within reference population in the before period using the gamma distribution and then extrapolating it for the AADT in the after period. It is assumed that if AADT changes in the
after period and no safety improvements are constructed, the percentile of safety performance within reference population of similar facilities will be preserved.

The percentile within reference population of the EB corrected safety performance is computed using the gamma distribution probability density function as follows:
$f(u)=\frac{a^{b} u^{b-1} e^{-a u}}{\Gamma(b)}$
$u$ - The mean for the facility
$\mu$ - The mean predicted by the SPF
$\alpha$ - Over-dispersion parameter estimated from the regression
$b$ - shape parameter $(b=1 / \alpha)$
$a-b / \mu$ (Scale parameter)
$\Gamma$ - Gamma Function

For instance if u = 6.23 crash/mi per year after correcting for the RTM in the before period and
$\mu=7.33 \frac{\text { crash }}{m i}$ per year, predicted by SPF
Gamma ( $\Gamma$ ) Function percentile (cumulative probability) can be computed as follows:
$\int_{u=0}^{u=6.23} \frac{a^{b} u^{b-1} e^{-a u}}{\Gamma(b)} d u=42.2 \%$
This computation is performed using Gamm Function (GAMMA.DIST) in the Excel spreadsheet (Figure 4) where

Alpha $=b$ (here $1 / \alpha=1 / 0.205=4.88)$ and Beta $=\mu / b$ (here $7.33 / 4.88=1.502$ )

```
GAMMA.DIST
```



Figure 4 Cumulative Probability of Gamma Function in Excel

Safety performance in the before period is represented by the 42.22 percentile of the reference population of similar facilities．AADT in the after period has increased to 71，366 which corresponds to the SPF mean $\mu=8.34 \frac{c r a s h}{m i}$ per year．Using Inverse Gamma Function（GAMMA．INV）in the Excel（Figure 5）we can now compute 42.22 percentile for the new mean of 8．34．The return of the Inverse Gamma Function at 42.22 percentile represents what safety performance would have been had safety improvement not been constructed，in this case $7.09 \frac{\mathrm{crash}}{\mathrm{mi}}$ per year．

Alpha $=b($ here $1 / 0.205=4.88)$ and Beta $=\mu / b($ here 8．34／4．88 $=1.709)$

| GAMMA．INV |  |  |  |
| ---: | :--- | :--- | :--- |
|  | Probability | 0.422 | 溷 |
|  | $=$ | 0.422 |  |
| Alpha | 4.88 | 溷 | $=4.88$ |
| Beta | 1.709 | 溷 | $=1.709$ |
|  |  |  | $=7.086478366$ |

Figure 5 Inverse Gamma Function for a Specified Percentile in Excel
6.23 crash／mile per year is what safety was in the before period and $7.08 \mathrm{crash} / \mathrm{mi}$ per year is what safety would have been had safety improvement not been constructed． Following construction observed safety performance in the after period resulted in 4.49 crash／mile per year．When compared with the 7.08 crash／mile per year it represents $\mathbf{3 6 . 5 8 \%}$ reduction in injury and fatal crashes．Figure 6 shows safety performance of I－76， MP 1．77－5．78 before（6．23），before without construction（7．09）and after（4．49）following construction on the same graph．


Figure 3 SPF Inj+Fat - I-76 (MP 1.77 to MP 5.78)
(EB Corrected Before Period, Before Without Construction, and After Period)

## HOW TO CONDUCT OBSERVATIONAL BEFORE AND AFTER STUDIES TO ESTIMATE CRASH MODIFICATION FACTORS

This section of the report represents a brief summary of the methodology described in the Federal Highway Administration's (FHWA) Guide to Developing Quality Crash Modification Factors ${ }^{3}$. It will first examine Before-After methodology using Comparison Group method followed by the review of the empirical Bayes Before-After methodology.

## BEFORE-AFTER WITH COMPARISON GROUP METHOD

A before-after with Comparison Group study uses an untreated comparison group of sites similar to the treated ones to account for changes in crashes unrelated to the treatment such as time and traffic volume changes. The Comparison Group is used to calculate the ratio of observed crash frequency in the after period to that in the before period. The observed crash frequency in the before period at a treatment site group is multiplied by this comparison ratio to provide an estimate of expected crashes at the treatment group if no treatment been applied. This is then compared to the observed crashes in the after period at the treatment site group to estimate the safety effect of the treatment. This method does not correct for regression-to-the mean bias, but it represents a simple alternative to the more complex empirical Bayes approach. It can be a useful strategy to evaluate the effectiveness of safety countermeasures when Safety Performance Functions for specific crash types are not available. The following example illustrates its application. Table 1 provides before and after crash counts for the treatment and comparison groups.

| Time Period | Treatment Group | Comparison Group |
| :---: | :---: | :---: |
| Before | 100 | 84 |
| After | 65 | 80 |

Table 1 Example Crash Count for before-After Comparison Group Study
The following terminology will be used:
Nobs, $, \mathrm{T}, \mathrm{B}=$ the observed number of crashes in the before period for the treatment group
Nobs.t.A. $=$ the observed number of crashes in the after period for the treatment group
Nobs.C.B $=$ the observed number of crashes in the before period for the comparison group $N_{\text {obs.c.A }}=$ the observed number of crashes in the after period for the comparison group

[^2]The Comparison Ratio $(C R)=N_{\text {obs.C.A }} / N_{\text {obs.C.B. }}$ It indicates how crash counts are expected to change in the absence of treatment. In this case $C R=80 / 84=0.9524$

Nexp. TA $=$ the expected number of crashes in the after period in the absence of treatment
$N_{\exp .}$ TA $=$ Nobs, $T, \mathrm{~B}$ CR $=100(0.9524)=95.24$
$\operatorname{Var}\left(\mathrm{N}_{\text {exp. }} \mathrm{TA}\right)=$ variance of the expected number of crashes in the after period
$\operatorname{Var}\left(\mathrm{N}_{\text {exp. }} \mathrm{TA}\right)=\mathrm{N}_{\text {exp. }} \mathrm{TA}^{2}\left(1 / \mathrm{N}_{\text {obs, }, \mathrm{B}, \mathrm{B}}+1 / \mathrm{N}_{\text {obs.C.B }}+1 / \mathrm{N}_{\text {obs.C. }}\right)=95.24^{2}\left(\frac{1}{100}+\frac{1}{84}+\frac{1}{80}\right)=$ 312.06

CMF = Crash Modification Factor
CMF $=\frac{N_{\text {obs }, T, A} / N_{\text {exp }, T, A}}{1+\operatorname{Var}\left(N_{\text {exp }, T, A}\right) /\left(N_{\text {exp }, T, A}^{2}\right)}=\frac{65 / 95.24}{1+312.06 / 95.4^{2}}=0.660$
$\operatorname{Var}(\mathrm{CMF})=$ variance of the CMF
$\operatorname{Var}(\mathrm{CMF})=\frac{C M F^{2}\left[\left(1 / N_{\text {obs }, T, A}\right)+\left(\operatorname{Var}\left(N_{\text {exp }, T, A} / N_{\text {exp }, T, A}^{2}\right)\right]\right.}{\left[1+\operatorname{Var}\left(N_{\text {exp }, T, A}\right) / N_{\text {exp }, T, A}^{2}\right]^{2}}=\frac{0.660^{2}\left[(1 / 65)+(312.06) /\left(95.24^{2}\right)\right]}{\left[1+(312.06) /(95.24)^{2}\right]^{2}}=0.0203$
Standard Error $(\sigma)=\sqrt{\operatorname{Var}(C M F)}=\sqrt{0.0203}=0.1424$
The cumulative probability factors for common confidence intervals are provided in Table 2.

| Confidence Interval | Cumulative Probability |
| :---: | :---: |
| $99 \%$ | 2.576 |
| $95 \%$ | 1.960 |
| $90 \%$ | 1.645 |

## Table 2 Cumulative Probability Factors

$95 \%$ Confidence Interval $=0.660 \pm 1.960(0.1424)$, which translates into a confidence interval of 0.381 to 0.939 . Note that that confidence interval does not contain 1 and therefore the results are statistically significant at the 95\% confidence level.

## EMPIRICAL BAYES BEFORE-AFTER METHOD

Similar to the comparison group method, the effect of the safety treatment is estimated by comparing the sum of the estimates of Nexp. TA for all treated sites with the number of crashes actually observed after treatment. The advantage of the empirical Bayes approach is that it correctly accounts for the changes in crash history that may be due to the regression-to-the-mean (RTM) phenomenon. RTM phenomenon reflects the tendency for random events, such as vehicle crashes to move toward the average during the course of an experiment or over time. The existence of the RTM bias has been long recognized and is now effectively addressed by using the Empirical Bayes (EB) method ${ }^{4}$. Additionally it provides a better approach than the comparison group method for accounting for changes in safety performance due to traffic volumes. The application of the empirical Bayes method requires the use of the Safety Performance Functions (SPF) and related over-dispersion parameters provided in the Colorado-specific safety knowledge base. Table 3 provides information to support example calculations using the empirical Bayes Before-After Method. For this simplified example, a weight (W) of 0.25 is assumed for the SPF prediction for all sites, and there are no traffic volume changes at the treated sites.

| Time Period | Treatment Group | SPF Estimates for <br> Treatment Group |
| :---: | :---: | :---: |
| Before | 100 | 81.08 |
| After | 65 | 81.08 |

## Table 3 Example Data for Empirical Bayes Before-After Study

Weight (W) provided in the problem statement is computed as follows:
$\mathrm{W}=\frac{1}{1+(\mu \times n) \alpha}=0.25$
Where
$\mu=$ Mean predicted by the SPF, here $\mathrm{N}_{\text {pred }, \mathrm{B}}=\mathrm{N}_{\text {pred }, \mathrm{A}}$ (no changes in traffic volume in this example)
$\mathrm{n}=$ number of years in the before or after period

[^3]$\alpha=$ Over-dispersion Parameter derived from SPF
The empirical Bayes estimate, $N_{\text {exp, }, \mathrm{T}, \mathrm{B}}$, is computed as:
$N_{\text {exp, }, \mathrm{T}, \mathrm{B}}=\mathrm{W} \mathrm{N}_{\text {pred }}+(1-\mathrm{W}) \mathrm{Nobs}, \mathrm{T}, \mathrm{B}=0.25(81.08)+(1-0.25) 100=95.27$
Since there was no changes in volume $\mathrm{N}_{\text {pred }, \mathrm{B}}=\mathrm{N}_{\text {pred, }, \mathrm{A}}$
$N_{\text {exp,T, }, ~}=95.27$
The variance of $\mathrm{Nexp}_{\mathrm{e}, \mathrm{T}, \mathrm{A}}$ is estimated as:
$\operatorname{Var}\left(\mathrm{N}_{\text {exp, }, \mathrm{T}, \mathrm{A}}\right)=\mathrm{N}_{\exp , \mathrm{T}, \mathrm{A}}(1-\mathrm{W})=95.27(1-0.25)=71.45$
$\mathrm{CMF}=\frac{N_{\text {obs }, T, A} / N_{\text {exp }, T, A}}{1+\operatorname{Var}\left(N_{\text {exp }, T, A}\right) /\left(N_{\text {exp }, T, A}^{2}\right)}=\frac{65 / 95.27}{1+71.45 / 95.7^{2}}=0.677$
$\operatorname{Var}(\mathrm{CMF})=\frac{C M F^{2}\left[\left(1 / N_{\text {obs }, T, A}\right)+\left(\operatorname{Var}\left(N_{\text {exp }, T, A}\right) / N_{\text {exp }, T, A}^{2}\right)\right]}{\left[1+\operatorname{Var}\left(N_{\text {exp }, T, A}\right) / N_{\text {exp }, T, A}^{2}\right]^{2}}=\frac{0.677^{2}\left[(1 / 65)+(71.45) /\left(95.27^{2}\right)\right]}{\left[1+(71.45) /(95.27)^{2}\right]^{2}}=$ $=0.0104$

Standard Error $(\sigma)=\sqrt{\operatorname{Var}(C M F)}=\sqrt{0.0104}=0.102$
In this case the results are statistically significant at the 99\% confidence level. $99 \%$ Confidence Interval $=0.677 \pm 2.576$ (0.102), which translates into 0.414 to 0.940 .

## APPENDIX B. SAFETY REPORTS

- \#15505 - US 550 Deer Fencing/Cattle Guards (MP 107.00-111.00)
- \#15645 - SH 83 Concrete Median (MP 69.39 - 70.57)
- \#15748 - I-76 Cable Rail (MP 1.77 - 5.78)
- \#15770 - I-25 Median Cable Rail (MP 102.50-107.50)
- \#15771 - SH 165 Guard Rail (MP 18.65-23.90)
- \#15828 - SH 93 / SH 72 Signal Upgrade - SH 93 Weather Related Road Closure System (MP 7.47-11.83)
- \#15861 - SH 52 / 95 ${ }^{\text {th }}$ Street Intersection Improvements (MP 3.16)
- \#15862 - US 34 / 11 $1^{\text {th }}$ Avenue Intersection Improvements (MP 112.23)
- \#15900 - SH 133 Guard Rail (MP 46.00 - 51.50)
- \#16005 - US 50 / Purcell Boulevard Intersection Improvements (MP 309.78)
- \#16006 - SH 45 / Red Creek Springs Intersection Improvements (MP 3.95)
- \#16010 - Industrial / Purcell New Signal


## Project Information

Project Name: US 550 near Ridgway State Park
Project Description: Install Double Cattle Guards and Extend Deer Fencing
CDOT Region: 5
Location: US 550

## Project Def: 15505

Mile Points: from 107 to 111
Work Start Date: 3/20/2007

County: Ouray
Length: 4 miles
Completion Date: 5/16/2008

Problem Description: As described in the Highway Safety Improvement Program (HSIP) application for this project, the ten-year crash history (1994-2003) showed that there were a total of 18 injury crashes ( 31 injuries), 50 PDO crashes, but no fatalities. This total included 23 wildlife related crashes. Much of the highway right of way (ROW) has deer fencing from approximately MP 106 to MP 113. The main entrance to Ridgway State Park is near MP 107, and it did not have any means to prevent wildlife from entering the ROW and being caught between the fencing along the corridor.

Improvement Description: Between March 20, 2007 and May 16, 2008, a double wildlife (cattle) guard was installed across the main entrance to the park and the existing deer fencing was extended to meet the new wildlife guard. It was anticipated that this would eliminate a primary entry point for wildlife to enter the highway ROW. The cost of construction was $\$ 295,155$.

The HSIP application anticipated that a 30\% reduction in all types of crashes might be realized by the improvement. The initial benefit/cost ratio was estimated to be 1.81 .

## Summary and Findings

The analysis of safety before and after the double wildlife (cattle) guard was installed as a barrier across the main entrance to the Ridgway State Park from US 550 showed an overall reduction in the wildlife type of crash that a wildlife guard is designed to mitigate. For this segment of 2-lane arterial highway, there were 44 total crashes during the five-year period before the wildlife guard was installed (2002-2006). In the five years after construction (2009 - 2013), the number of crashes decreased to 28. This decrease in crashes was accompanied by a modest increase in AADT reflected by the frequency SPF. In addition, the number fatal crashes also diminished although the number of injury crashes (and injuries) remained the same.

A comparison of wildlife type crashes before and after the double wildlife (cattle) guard barrier improvement was installed showed that there was an increase in injury crashes (from 1 INJ in 5 years before to 2 INJ in the 5 years after). The number of PDO crashes was reduced from 19 to 12. The ratio of benefits and cost for this project shows that benefits are outweighed by costs as the $\mathrm{B} / \mathrm{C}$ ratio is 0.24 to one. The result is an improvement that might not have been justified from an economic standpoint since the decrease in the number of PDO crashes is outweighed by the unfortunate increase in injury crashes, although the total number of wildlife crashes did decrease.

## Results of Safety Analyses

Using Vision Zero Suite, the review of before and after crash records shows a decrease in the number of crashes; the total number of crashes decrease from 44 during the five-year period (2002 to 2006) before the wildlife barrier project was constructed (see Table 1 and Exhibit 1) to 28 during the five-year after period (2009 to 2013) (see Table 1 and Exhibit 2). The number of serious crashes showed a decrease in that there was no fatality during the after period:

- Before (2002 - 2006) - 1 fatal crash with 1 fatality (sideswipe opposite) and 9 injury crashes with 13 injuries
- After (2009-2013) - no fatal crashes and 9 injury crashes with 13 injuries

This decrease in the total number of crashes occurred in spite of a modest increase in traffic volumes on US 550: 6,500 vehicles per day (vpd) for the before period and 7,140 vpd in the after period reflected by the SPF analysis.

Table 1 - Results of Overall Crash Analyses

| SH 550 MP 107-111 | Before | After |
| :---: | :---: | :---: |
| Time Period: | 1/1/2002 to 12/31/2006 (5 yr.) | 1/1/2009 to 12/31/2013 (5 yr.) |
| AADT | 6,488 vpd | 7,140 vpd |
| Filters: | None | None |
| Total Crashes | 44 | 28 |
| Fatal Crashes (Fatalities) | 1 (1) | 0 |
| Injury Crashes (Injuries) | 9 (13) | 9 (13) |
| Property Damage Only | 34 | 19 |
| Crash Types: \# (\%) [significance] |  |  |
| Wild Animal | 19 (43.2\%) [99.96] | 14 (50.0\%) [99.26\%] |
| Fixed Objects | 10 (22.7\%) [97.02\%] | 8 (28.6\%) |
| Overturning | 5 (11.4\%) | 1 (3.6\%) |
| Rear End | 3 (6.8\%) | 2 (7.1\%) |
| Sideswipe Same | 3 (6.8\%) | 1 (3.6\%) |
| Sideswipe Opposite | 2 (4.5\%) | 0 |
| Fixed Object Crashes: \# (\% of FO) [significance] |  |  |
| Fence | 4 (40.0\%) | 0 |
| Tree | 4 (40.0\%) | 2 (25.0\%) |
| Sign | 1 (10.0\%) | 1 (12.5\%) |
| Large Boulder/Rock | 1 (10.0\%) | 3 (37.5\%) |
| Embankment | 0 | 2 (25.0\%) |

The magnitude of safety problems on select highway sections and intersections can be assessed through the use of Safety Performance Function (SPF) methodology. A SPF reflects the complex relationship between exposure (measured in ADT) and the crash count for a section of roadway measured in crashes per mile per year (CPMPY) or for an intersection, measured in crashes per year. The SPF models provide an estimate for the expected crash frequency and severity for a range of ADT among similar facilities. This allows for an assessment of the magnitude of the safety problem from a frequency standpoint.

Development of the SPF lends itself well to the conceptual formulation of the Levels of Service of Safety (LOSS). The concept of level of service uses qualitative measures that characterize safety of a roadway segment in reference to its expected performance and severity. If the level of safety predicted by the SPF represents a normal or expected number of crashes at a specific level of ADT, then the degree of deviation from the normal can be stratified to represent specific levels of safety.

LOSS-I - Indicates low potential for crash reduction
LOSS-II - Indicates low to moderate potential for crash reduction
LOSS-III - Indicates moderate to high potential for crash reduction
LOSS-IV - Indicates high potential for crash reduction
LOSS boundaries are calibrated by computing the $20^{\text {th }}$ and the $80^{\text {th }}$ percentiles using the Gamma Distribution Probability Density Function. Gradual change in the degree of deviation of the LOSS boundary line from the fitted model mean reflects the observed increase of variability in crashes as ADT increases. LOSS reflects how a segment of roadway or intersection is performing in regard to its expected crash frequency at a specific level of ADT.

SPF plots for both total crashes (see Figure 1) and for fatal and injury crashes (see Figure 2) also reflect this improvement in the crash record. LOSS improved from the LOSS III range for total crashes in the before period to LOSS II in the after period. Injury/Fatal crashes improved in the after period, although still within the LOSS II range (see Table 2), due to the absence of a fatal crash. However, it is difficult to conclude that the overall decrease in almost all types of crashes (except wildlife) can be attributed solely to the installation of the double wildlife (cattle) guard at the main park entrance. Figures 1 and 2 also show that the number of crashes during the period after construction was much improved in comparison to what it could have been without the project.

Figure 1 - SPF for Total Crashes
US 550 (MP 107 to MP 111)
Before: $\mathbf{2 0 0 2}$ to 2006 After: 2009 to 2013


Note: Safety Performance Function (SPF) Model: Colorado - Rural Flat and Rolling 2-Lane Undivided Highway

Figure 2 - SPF for Injury and Fatal Crashes
US 550 (MP 107 to MP 111)
Before: $\mathbf{2 0 0 2}$ to $\mathbf{2 0 0 6}$ After: $\mathbf{2 0 0 9}$ to 2013


Note: Safety Performance Function (SPF) Model: Colorado - Rural Flat and Rolling 2-Lane Undivided Highway

Table 2 - Safety Performance Function (SPF)

| SH 550 MP 107-111 | Before | After | No Build After |
| :--- | :---: | :---: | :---: |
| EB Correction: | Yes | No | Yes |
| SPF Graph | Rural, Flat \& Rolling, <br> 2-lane Undivided <br> Highway |  <br> Rolling, 2-lane <br> Undivided Highway | Rural, Flat \& Rolling, <br> 2-lane Undivided <br> Highway |
| Total Crashes: | LOSS III | LOSS II | LOSS III |
| LOSS | 2.13 | 1.40 | 2.30 |
| CPMPY | 1.81 | 1.95 | 1.95 |
| Mean CPMPY | 1.177 | 0.718 | 1.177 |
| Proportion of Mean |  |  |  |
| Fatal \& Injury Crashes: | LOSS II | LOSS II | LOSS II |
| LOSS | 0.60 | 0.45 | 0.68 |
| CPMPY | 0.70 | 0.77 | 0.77 |
| Mean CPMPY | 0.857 | 0.584 | 0.857 |
| Proportion of Mean |  |  |  |

A more detailed review of the before and after crash record reveals that a somewhat mixed improvement in safety can be attributed to the installation of the double wildlife (cattle) guard. Table 3 provides a comparison of the wildlife type crash that is most directly affected by the new guard installation. The No Build After crashes were estimated using the increase in the median of the SPF for total crashes found in Table 2 (increase is $1.077=1.95 / 1.81$ ). Table 3 shows an increase in injury crashes (from 1 in 5 years before to 2 in the 5 years after). The number of PDO crashes was reduced from 19 to 12.

Table 3 - Results of Wildlife Crash Analyses

| SH 550 MP 107-111 | Before | After | No Build After |
| :---: | :---: | :---: | :---: |
| Time Period: | $\begin{gathered} \hline 1 / 1 / 2002 \text { to } \\ 12 / 31 / 2006 \text { ( } 5 \text { yr.) } \end{gathered}$ | $\begin{gathered} \hline \text { 1/1/2009 to } \\ 12 / 31 / 2013 \text { (5 yr.) } \end{gathered}$ | $\begin{gathered} \text { 1/1/2009 to } \\ 12 / 31 / 2013 \text { (5 yr.) } \end{gathered}$ |
| Crash Types: |  |  |  |
| Wildlife - Total | 19 | 14 | 20 |
| Injury (injuries) | 1 (1) | 2 (2) | 1 (1) |
| PDO | 18 | 12 | 19 |
| \% Reduction in Total |  |  |  |

Vision Zero Suite includes benefit/cost (B/C) analyses within its procedures. The results of the B/C analysis are shown in Exhibit 3 for wildlife type crashes. The increase in injury crashes in the after period was factored into the analysis by increasing the cost of construction for the wildlife (cattle) guard. Over the design life of 10 years for the guard, the increased cost of crashes would be $\$ 161,400(2 \mathrm{INJ}=2 \mathrm{X} \$ 80,700)$. Exhibit 3 shows the result of the Benefit/Cost calculation is a $\mathrm{B} / \mathrm{C}$ ratio of 0.24 . This result shows that the project might not have been justified from an economic standpoint since the decrease in the number of PDO crashes is outweighed by the unfortunate increase in injury crashes, although the total number of wildlife crashes did decrease.

## Exhibit 3 - Benefit Cost Analysis - Wildlife Crashes Only




## ADT: 6,488 Length: 3.97



## ADT: 6,488 Length: 3.97



## ADT: $7,140 \quad$ Length: 3.99



## ADT: 7,140 Length: 3.99

## Project Information

Project Name: $\quad$ SH 83A (Parker Road) from Lehigh Avenue to I-225A
Project Description: Install Concrete Median Barrier

CDOT Region: 6
Location: SH 83A
Schedule:

## Project Def: 15645

Mile Points: from 69.39 to 70.57
Work Start Date: 9/11/2006

County: Arapahoe

Length: 1.18 miles
Completion Date: 1/17/2007

Problem Description: As described in the Highway Safety Improvement Program (HSIP) application for this project, the five-year crash history (2000 - 2004) showed a number of headon, sideswipe in opposite direction, median crossover, and off median/left type crashes. The number of head-on crashes (9) was higher than expected. These crashes occurred with a high severity ( 1 fatal and 10 injury crashes).

Improvement Description: Between September 11, 2006 and January 17, 2007, a concrete median barrier ( 1.18 miles) was constructed between the intersection at Lehigh Avenue and the intersection at l-225. (There may have been short segments of concrete median barrier in place before this project). This barrier was installed to reduce the potential for head-on and sideswipe (opposite direction) crashes. The cost of construction was $\$ 1,320,726$.

The HSIP application anticipated that the following reductions in crashes might be realized by the improvement anticipated: fatal crashes - 60\%, injury crashes - 40\%, and property damage only $-0 \%$. The initial benefit/cost ratio was estimated to be 2.21 .

## Summary and Findings

The analysis of safety before and after the concrete median barrier was installed along SH 83A between Lehigh Avenue and I-225 showed reductions in the types of crashes that a median barrier is designed to mitigate. For this segment of limited access highway, there were 229 total crashes (mainline, non-intersection) during the five-year period before the concrete barrier was installed (2001-2005). In the five years after construction (2008-2012), the number of crashes increased slightly to 240 . Since daily volumes continued to increase throughout the study period, the crash rate was reduced. In addition, the number of injury and fatal crashes also diminished.

The concrete median barrier improvement was directly responsible for decreases in the number and severity of head-on, overturning and sideswipe (opposite) crashes. During the before period, there was one fatal head-on collision and two injury crashes that involved injuries to 4 people. The after period experienced no fatal or injury crashes of these three types. The number of crashes involving the concrete median barrier remained the same in the before and after periods, although the number of injury crashes was reduced.

The ratio of benefits derived from crash reduction to the cost of construction for this project shows that benefits outweigh costs by a ratio of 5.91 to one. The result is an improvement that was certainly justified, especially since there were no fatal crashes in the period after construction.

## Results of Safety Analyses

Using Vision Zero Suite, the review of before and after crash records shows a slight increase in the number of crashes; the total number of crashes increased from 229 during the five-year period (2001 to 2005) before the concrete median barrier project was constructed (see Table 1 and Exhibit 1) to 240 during the five-year after period (2008 to 2012) (see Table 1 and Exhibit 2). As identified in Table 1, these crashes were not at either of the intersections in the study area and involved the mainline of SH 83A only. The number of serious crashes showed a more significant decrease:

- Before (2001 - 2005) - 2 fatal crashes with 2 fatalities (1 head-on and 1 involving a pedestrian) and 74 injury crashes with 98 injuries
- After (2008-2012) - no fatal crashes and 68 injury crashes with 90 injuries

This decrease in severe crashes occurred in spite of a modest increase in traffic volumes on SH 83A: 68,600 vehicles per day (vpd) estimated for the before period and $73,750 \mathrm{vpd}$ in the after period. This combination of increased traffic and decreased number of crashes also resulted in a decrease in the accident rates:

- Before (2001 - 2005): 1.55 crashes per million vehicle miles of travel (cpmvmt)
- After (2008 - 2012): 1.49 (cpmvmt)

Table 1 - SH 83A (MP 69.39 to MP 70.57) - Results of Overall Crash Analyses

|  | Before | After |
| :---: | :---: | :---: |
| Time Period: | 1/1/2001 to 12/31/2005 (5 yr.) | 1/1/2008 to 12/31/2012 (5 yr.) |
| AADT | 68,579 vpd | 73,749vpd |
| Filters: | Non-Intersection / Mainline Only | Non-Intersection / Mainline Only |
| Total Crashes | 229 | 240 |
| Fatal Crashes (Fatalities) | 2(2) | 0 |
| Injury Crashes (Injuries) | 74 (98) | 68 (90) |
| Property Damage Only | 153 | 172 |
| Crash Types: \# (\%) |  |  |
| Rear End | 122 (53.3\%) | 156 (65.0\%) |
| Sideswipe Same | 45 (19.7\%) | 43 (25.3\%) |
| Fixed Objects | 37 (16.2\%) | 27 (11.2\%) |
| Head-On | 6 (2.6\%) | 0 |
| Overturning | 2(0.9\%) | 3 (1.2\%) |
| Sideswipe Opposite | 4 (1.7\%) | 0 |
| Other Objects | 3(1.3\%) | 5 (2.0\%) |
| Fixed Object Crashes: \# (\% of FO) |  |  |
| Concrete Barrier | 7 (18.9\%) | 16 (59.6\%) |
| Guard Rail | 16 (43.2\%) | 4 (14.8\%) |
| Curb | 3 (8.1\%) | 0 |
| Crash Cushion | 3 (8.1\%) | 2 (7.4\%) |
| Tree | 1 (2.7\%) | 2 (7.4\%) |

Normally, the magnitude of safety problems on highway sections and intersections can be assessed through the use of Safety Performance Function (SPF) methodology. However in the case of SH 83A which is an urban arterial, no SPF can be developed. Additionally, no crash pattern norms for diagnostic analyses are available for the same reason.

A more detailed review of the before and after crash record reveals that a significant improvement in safety can be attributed to the installation of the concrete median barrier. Table 2 shows a comparison of four types of crashes (all off-road left or off median) that are most directly affected by the barrier: head-on, sideswipe (opposite), overturning, and hitting the concrete barrier itself. The No Build After crashes were estimated using the increase in the average daily traffic volumes found in Table 1 (increase is $1.075=73,749 / 68,579$ ).

Table 2 - SH 83A (MP 69.39 to MP 70.57) - Results of Concrete Median Barrier Crash Analyses

|  | Before | After | No Build After |
| :---: | :---: | :---: | :---: |
| Time Period: | $\begin{gathered} \hline \text { 1/1/2001 to } \\ 12 / 31 / 2005 \text { (5 yr.) } \end{gathered}$ | $\begin{gathered} \hline \text { 1/1/2008 to } \\ 12 / 31 / 2012 \text { (5 yr.) } \end{gathered}$ | $\begin{gathered} \hline \text { 1/1/2008 to } \\ 12 / 31 / 2012 \text { (5 yr.) } \end{gathered}$ |
| Crash Types: |  |  |  |
| Head-On - Total | 6 | 0 | 6 |
| Fatal (fatalities) | 1 (1) | 0 | 1 (1) |
| Injury (injuries) | 3 (6) | 0 | 3 (6) |
| PDO | 1 | 0 | 1 |
| \% Reduction in Total |  | 100\% |  |
| Sideswipe (Opp.)- Total | 4 | 0 | 4 |
| Injury (injuries) | 2 (4) | 0 | 2 (4) |
| PDO | 2 | 0 | 2 |
| \% Reduction in Total |  | 100\% |  |
| Overturning - Total (off-left/off-median only) | 1 | 0 | 1 |
| Injury (injuries) | 0 | 0 | 0 |
| PDO | 1 | 0 | 1 |
| \% Reduction in Total |  | 100\% |  |
| Concrete Barrier - Total (off-left/off-median only) | 7 | 7 | 7 |
| Injury (injuries) | 4 (4) | 1 (1) | 4 (4) |
| PDO | 3 | 6 | 3 |

Vision Zero Suite includes benefit/cost (B/C) analyses within its procedures. The results of the B/C analysis are shown in Exhibit 3 for head-on, sideswipe (opposite), and overturning crashes and Exhibit 4 for concrete barrier crashes. The increase in property damage only crashes from hitting the concrete barrier was factored into the analysis by increasing the cost of construction for concrete median barrier crashes. During the 5 -year after period, there were 6 PDO concrete
median barrier crashes, up from the 3 PDO crashes that occurred in the before period. Over the design life of 20 years for the concrete barrier, the increased cost of crashes would be $\$ 111,600$ (12 PDO $=12 \times \$ 9,300$ ). Since there were concrete barrier crashes in the before period, it can be speculated that the study section of SH 83A had short segments of concrete median barrier that caused a higher level of injury crashes. The barrier does not eliminate new crashes since it creates a barrier near the roadway, but the result is a trade-off of less serious crashes (PDOs primarily) instead of more serious injury crashes.

Exhibits 3 and 4 provide the Benefit/Cost calculations. The $\mathrm{B} / \mathrm{C}$ ratio for eliminating head-on, sideswipe (opposite), and overturning crashes is 5.40 , and the B/C ratio for concrete barrier crashes is 0.51 . Therefore, the resulting B/C ratio for the cable rail project is $5.91(5.40+0.51)$, showing that the improvement was certainly justified, especially since there were no fatal crashes in the period after construction.

## Exhibit 3 - Benefit Cost Analysis - SH 83A (MP 69.39 to MP 70.57) - Head-on, Sideswipe (Opposite), and Overturning Crashes Only



## Exhibit 4 - Benefit Cost Analysis - SH 83A (MP 69.39 to MP 70.57) - Concrete Barrier Only




## ADT: 68,579 Length: 1.18



## ADT: 68,579 Length: 1.18



## ADT: 73,749 Length: 1.19



## Project Information

Project Name: I-76 - Sheridan to I-25
Project Description: Install Tensioned Cable Barrier
CDOT Region: 6
Location: I-76

## Project Def: 15748

Mile Points: from: 1.77 to 5.78
Work Start Date: 7/9/2007

County: Adams
Length: 4.01 miles
Completion Date: 10/19/2007

Problem Description: As described in the Highway Safety Improvement Program (HSIP) application for this project, the five-year crash history showed a higher than expected number of head-on, sideswipe in opposite direction, and overturning type crashes. These crashes were occurring off-left and in the median. These crashes occurred with a higher than expected severity ( 5 fatal and 3 injury crashes).

Improvement Description: In 2007, cable guard rail ( 2.58 miles) was installed at locations where vehicles have the potential to leave the road to the left and perhaps cause head-on, sideswipe, or overturning-type crashes. The cost of construction was $\$ 521,450$.

The HSIP application anticipated that three crash types would be impacted by this improvement: off median head-on, off left overturning, and sideswipe opposite. The following reductions in crashes were anticipated: fatal crashes - 60\%, injury crashes - 40\%, and property damage only $-20 \%$. The initial benefit/cost ratio was estimated to be 14.36.

## Summary and Findings

The analysis of safety before and after the cable guard rail was installed along I-76 between Sheridan Boulevard and I-25 showed significant safety improvements. For this segment of freeway, there were 355 total crashes during the five-year period before the rail was installed (2002-2006). In the five years after construction (2008-2012), the number of crashes was decreased to 348 . Since daily volumes continued to increase throughout the study period, the crash rate was reduced. In addition, the number of injury and fatal crashes also diminished.

The cable rail improvement was directly responsible for decreases in the number and severity of head-on and overturning crashes. During the before period, there was one fatal head-on collision and a number of injury crashes. The after period experienced no fatal crashes and a reduction in the number of injury crashes. Cable rail causes new crashes since it creates a barrier near the roadway, but the result is a trade-off of less serious crashes (PDOs primarily) instead of more serious fatal and injury crashes.

The ratio of benefits and cost for this project shows that benefits outweigh costs by a ratio of 6.16 to one. The result is an improvement that was certainly justified.

## Results of Safety Analyses

Using VZS, the review of before and after crash records shows a slight decrease in the number of crashes; the total number of crashes decreased from 355 during the five-year period ( 2002 to 2006) before the cable rail project was constructed (see Table 1 and Exhibit 1) to 348 during the five-year after period (2008 to 2012) (see Table 1 and Exhibit 2). The number of serious crashes showed a more significant decrease:

- Before (2002 - 2006) - 3 fatal crashes with 4 fatalities (1 head-on and 2 involving guard rail) and 122 injury crashes with 166 injuries
- After (2008-2012) - no fatal crashes and 92 injury crashes with 112 injuries

This decrease in severe crashes occurred in spite of a modest increase in traffic volumes on I76: 63,800 vehicles per day ( vpd ) estimated for the before period and $71,400 \mathrm{vpd}$ in the after period. This combination of increased traffic and decreased number of crashes also resulted in a decrease in the accident rates:

- Before (2002 - 2006): 0.74 crashes per million vehicle miles of travel (cpmvmt)
- After (2008-2012): 0.65 (cpmvmt)

Table 1 - I-76 (MP 1.77 to MP 5.78) - Results of Overall Crash Analyses

|  | Before | After |
| :---: | :---: | :---: |
| Time Period: | 1/1/2002 to 12/31/2006 (5 yr.) | 1/1/2008 to 12/31/2012 (5 yr.) |
| AADT | 63,800 vpd (est.) | 71,366 vpd |
| Filters: | Mainline Only/Non-Intersection | Mainline Only/Non-Intersection |
| Total Crashes | 355 | 348 |
| Fatal Crashes (Fatalities) | 3 (4) | 0 |
| Injury Crashes (Injuries) | 122 (166) | 92 (112) |
| Property Damage Only | 230 | 256 |
| Crash Types: \# (\%) [significance] |  |  |
| Fixed Objects | 114 (32.1\%) | 121 (34.8\%) |
| Rear End | 104 (29.3\%) | 78 (22.4\%) |
| Sideswipe Same | 67 (18.9\%) [99.60\%] | 88 (25.3\%) [100.00\%] |
| Overturning | 27 (7.6\%) | 18 (5.2\%) |
| Head-On | 6 (1.7\%) | 2 (0.6\%) |
| Other Objects | 22 (6.2\%) | 33 (9.5\%) |
| Fixed Object Crashes: \# (\%) [significance] |  |  |
| Concrete Barrier | 50 (43.9\%) [100.00\%] | 53 (43.8\%) [99.99\%] |
| Cable Rail | n/a | 26 (21.5\%) [100.00\%] |
| Guard Rail | 18 (15.8\%) | 16 (13.2\%) |
| Fence | 15 (13.2\%) [100.00\%] | 6 (5.0\%) |
| Bridge Rail | 6 (5.3\%) | 0 |
| Light/Utility Pole | 8 (7.0\%) [99.86\%] | 6 (5.0\%) |

The magnitude of safety problems on select highway sections and intersections can be assessed through the use of Safety Performance Function (SPF) methodology. A SPF reflects the complex relationship between exposure (measured in ADT) and the crash count for a section of roadway measured in crashes per mile per year (CPMPY) or for an intersection, measured in crashes per year. The SPF models provide an estimate for the expected crash frequency and severity for a range of ADT among similar facilities. This allows for an assessment of the magnitude of the safety problem from a frequency standpoint.

Development of the SPF lends itself well to the conceptual formulation of the Levels of Service of Safety (LOSS). The concept of level of service uses qualitative measures that characterize safety of a roadway segment in reference to its expected performance and severity. If the level of safety predicted by the SPF represents a normal or expected number of crashes at a specific level of ADT, then the degree of deviation from the normal can be stratified to represent specific levels of safety.

LOSS-I - Indicates low potential for crash reduction
LOSS-II - Indicates low to moderate potential for crash reduction
LOSS-III - Indicates moderate to high potential for crash reduction
LOSS-IV - Indicates high potential for crash reduction
Gradual change in the degree of deviation of the LOSS boundary line from the fitted model mean reflects the observed increase of variability in crashes as ADT increases. LOSS reflects how a segment of roadway or intersection is performing in regard to its expected crash frequency at a specific level of ADT.

SPF plots for both total crashes (see Figure 1) and for fatal and injury crashes (see Figure 2) also reflect this improvement in the crash record. LOSS improved within the LOSS II range for total crashes in the after period, and Injury/Fatal crashes improved to LOSS I in the after period (see Table 2), due to the decrease in both types of severe crashes. However, it is difficult to conclude that the overall decrease in almost all types of crashes (except cable rail) can be attributed solely to the installation of the cable rail in the median of I-76 during 2007. However, as will be discussed in the following section, the cable rail has significantly reduced certain types of crashes.

A more detailed review of the before and after crash record reveals that a significant improvement in safety can be attributed to the installation of the median cable rail. Table 3 shows a comparison of three types of crashes that are most directly affected by the cable rail: head-on, overturning (off-road left and off median), and hitting the cable rail. The No Build After crashes were estimated using the increase in the median of the SPF for total crashes found in Table 2 (increase is $1.173=26.38 / 22.49$ ). Figures 1 and $\mathbf{2}$ also show that the number of crashes after construction was much better than it could have been without the project.

Figure 1 -SPF for Total Crashes
I-76 (MP 1.77 to MP 5.78)
Before: 2002 to 2006 After: 2008 to 2012


Note: Safety Performance Function (SPF) Model: Colorado - Urban Flat Rolling Mountainous 4-Lane Divided Freeways (Revised

Figure 2-SPF for Injury and Fatal Crashes
I-76 (MP 1.77 to MP 5.78)
Before: 2002 to 2006 After: 2008 to 2012


Note: Safety Performance Function (SPF) Model: Colorado - Urban Flat Rolling Mountainous 4-Lane Divided Freeways (Revised)

Table 2 - I-76 (MP 1.77 to MP 5.78) - Safety Performance Function (SPF)

|  | Before | After | No Build After |  |
| :--- | :---: | :---: | :---: | :---: |
| EB Correction: | Yes | No | Yes |  |
| SPF Graph | Urban, 4-lane <br> Freeway | Urban, 4-lane <br> Freeway | Urban, 4-lane <br> Freeway |  |
| Total Crashes: | LOSS II | LOSS II | LOSS II |  |
| LOSS | 17.49 | 16.98 | 20.52 |  |
| CPMPY | 22.49 | 26.38 | 26.38 |  |
| Mean CPMPY | 0.778 | 0.644 | 0.778 |  |
| Proportion of Mean |  |  |  |  |
| Fatal \& Injury Crashes: | LOSS II | LOSS I | LOSS II |  |
| LOSS | 6.23 | 4.49 | 7.09 |  |
| CPMPY | 7.33 | 8.34 | 8.34 |  |
| Mean CPMPY | 0.850 | 0.538 | 1.275 |  |
| Proportion of Mean |  |  |  |  |

Vision Zero Suite (VZS) includes benefit/cost (B/C) analyses within its procedures. The results of the B/C analysis are shown in Exhibit 3 for overturning crashes and Exhibit 4 for head-on crashes. The increase in cable rail crashes was factored into the analysis by increasing the cost of construction for head-on crashes. During the 5 -year after period, there were 2 injury and 22 PDO cable rail crashes. Over the design life of 20 years for the cable rail system, the increased cost of crashes would be \$1,936,000 (88 PDO = \$783,000 and 8 injuries = \$631,000). Cable rail causes new crashes since it creates a barrier near the roadway, but the result is a trade-off of less serious crashes (PDOs primarily) instead of more serious fatal and injury crashes.

Exhibits 3 and 4 provide the Benefit/Cost calculations. The B/C ratio for overturning crashes is 3.53 and the $B / C$ ratio for head-on crashes is 2.53 . Therefore, the resulting $B / C$ ratio for the cable rail project is $6.16(3.63+2.53)$, showing that the improvement was certainly justified.

Table 3 - I-76 (MP 1.77 to MP 5.78) - Results of Off-Road Left and Off-Median Crash Analyses

|  | Before | After | No Build After |
| :---: | :---: | :---: | :---: |
| Time Period: | $\begin{gathered} 1 / 1 / 2002 \text { to } \\ 12 / 31 / 2006 \text { (5 yr.) } \end{gathered}$ | $\begin{gathered} \hline 1 / 1 / 2008 \text { to } \\ 12 / 31 / 2012 \text { (5 yr.) } \\ \hline \end{gathered}$ | $\begin{gathered} \hline 1 / 1 / 2008 \text { to } \\ 12 / 31 / 2012 \text { (5 yr.) } \\ \hline \end{gathered}$ |
| Crash Types: |  |  |  |
| Overturning - Total | 12 | 2 | 14 |
| Injury (injuries) | 7 (8) | 1 (1) | 8 (9) |
| PDO | 5 | 1 | 6 |
| \% Reduction in Total (INJ / PDO) |  | 89 / 83\% |  |
| Head-On - Total | 5 | 0 | 6 |
| Fatal (fatalities) | 1 (1) | 0 | 1 (1) |
| Injury (injuries) | 2 (2) | 0 | 2 (2) |
| PDO | 2 | 0 | 3 |
| \% Reduction in Total |  | 100\% |  |
| Cable Rail - Total | n/a | 24 | n/a |
| Injury (injuries) | n/a | 2 (2) | n/a |
| PDO | n/a | 22 | n/a |

Exhibit 3 - Benefit Cost Analysis - I-76 (MP 1.77 to MP 5.78) - Overturning Crashes Only


## Exhibit 4 - Benefit Cost Analysis - I-76 (MP 1.77 to MP 5.78) - Head-On and Cable Rail Crashes




## ADT: 44,095 Length: 4.10



## ADT: 44,095 Length: 4.10



## ADT: 71,366 Length: 4.09



## ADT: 71,366 Length: 4.09

## Project Information

Project Name: I-25 Median Cable Rail
Project Description: Install cable rails along I-25 N/O Pueblo

CDOT Region: 2
Location: I-25
Schedule:

## Project Def: 15770

Mile Points: 102.5 - 107.5
Work Start Date: 6/25/2007

County: Pueblo

Length: 5.0 miles
Completion Date: 1/23/2008

Problem Description: As described in the Highway Safety Improvement Program (HSIP) application for this project, the five-year crash history showed a higher than expected number of off-road type crashes. I-25 southbound and northbound lanes are separated by a median width of 22 feet or less. The median is grass and has very little depression. The clear zone recommendation is 26 feet which would mean a 52 foot median. Also in this stretch, the frontage road is close to the highway with some locations having grade separation.

Improvement Description: In 2007, cable rail was installed along the entire stretch of median and on the shoulder between I-25 and the frontage road in select locations. The cost of construction was $\$ 1,042,820$.

The HSIP application anticipated that off-road crash types would be impacted by this improvement. The following reductions in crashes were anticipated: fatal crashes $-60 \%$, injury crashes $-40 \%$, and property damage only $-20 \%$. The initial benefit/cost ratio was estimated to be 2.65 .

## Summary and Findings

The analysis of safety performance of I-25 MP 102.5-107.5 before and after the cable rail was installed showed some safety improvements. For this segment of highway, there were 163 total crashes during the five-year period before the rail was installed (2002-2006). In the five years after construction ( $7 / 2008-6 / 2013$ ), the number of crashes was decreased to 151. In addition, the number of injury and fatal crashes also diminished.

The cable rail improvement was directly responsible for decreases in the number and severity of off-road crashes. During the before period, there was 4 fatal crashes and a number of injury crashes. The after period experienced 3 fatal crashes and a reduction in the number of injury crashes. Cable rail causes new crashes since it creates a barrier near the roadway, but the result is a trade-off of less serious crashes (PDOs primarily) instead of more serious fatal and injury crashes.

The ratio of benefits and cost for this project shows that benefits outweigh costs by a ratio of 2.26 to one. The result is an improvement that was justified.

## Results of Safety Analyses

Using VZS, the review of before and after crash records shows a decrease in the number of crashes; the total number of crashes decreased from 163 during the five-year period ( 2002 to 2006) before the cable rail was installed (see Table 1 and Exhibit 1) to 151 during the five-year after period (7/2008 to 6/2013) (see Table 1 and Exhibit 2). The number of serious crashes also showed a decrease:

- Before (2002 - 2006) - 4 fatal crashes with 4 fatalities (2 overturning, head-on, embankment) and 45 injury crashes with 74 injuries
- After (7/2008 - 6/2013) - 3 fatal crashes with 3 fatalities ( 2 pedestrian, overturning) and 42 injury crashes with 61 injuries

This decrease in crashes occurred along with an increase in traffic volumes on I-25: 29,000 vehicles per day ( vpd ) for the before period and $30,100 \mathrm{vpd}$ in the after period is reflected by the SPF analysis.

Table 1 - Results of Overall Crash Analyses

|  | Before | After |
| :---: | :---: | :---: |
| Time Period: | 1/1/2002 to 12/31/2006 (5 yr.) | 7/1/2008 to 6/30/2013 (5 yr.) |
| AADT | 28,989 vpd | 30,109 vpd |
| Filters: | Mainline- Only Non-Intersection | Mainline Only Non-Intersection |
| Total Crashes | 163 | 151 |
| Fatal Crashes (Fatalities) | 4 (4) | 3 (3) |
| Injury Crashes (Injuries) | 45 (74) | 42 (61) |
| Property Damage Only | 114 | 106 |
| Crash Types: \# (\%) [significance] |  |  |
| Fixed Objects | 47 (28.8\%) [99.32\%] | 76 (50.3\%) [100.00\%] |
| Overturning | 25 (15.3\%) [100.00\%] | 17 (11.3\%) [99.93\%] |
| Rear End | 23 (14.1\%) | 11 (7.3\%) |
| Other Objects | 22 (13.5\%) [100.00\%] | 9 (6.0\%) |
| Sideswipe Same | 17 (10.4\%) | 24 (15.9\%) [99.85\%] |
| Head-On | 2 (1.2\%) | 1 (0.7\%) |
| Fixed Object Crashes: \# (\%) [significance] |  |  |
| Guard Rail | 13 (27.7\%) [99.78\%] | 29 (38.2\%) [100.00\%] |
| Fence | 10 (21.3\%) [100.00\%] | 7 (9.2\%) |
| Delineator Post | 9 (19.1\%) [99.97\%] | 3 (3.9\%)0 |
| Embankment | 5 (10.6\%) | 4 (5.3\%) |
| Sign | 3 (6.4\%) | 9 (11.8\%) |
| Cable Rail | n/a | 21 (27.6\%) [100.00\%] |

The magnitude of safety problems on select highway sections and intersections can be assessed through the use of Safety Performance Function (SPF) methodology. A SPF reflects the complex relationship between exposure (measured in ADT) and the crash count for a section of roadway measured in crashes per mile per year (CPMPY) or for an intersection,
measured in crashes per year. The SPF models provide an estimate for the expected crash frequency and severity for a range of ADT among similar facilities. This allows for an assessment of the magnitude of the safety problem from a frequency standpoint.

Development of the SPF lends itself well to the conceptual formulation of the Levels of Service of Safety (LOSS). The concept of level of service uses qualitative measures that characterize safety of a roadway segment in reference to its expected performance and severity. If the level of safety predicted by the SPF represents a normal or expected number of crashes at a specific level of ADT, then the degree of deviation from the normal can be stratified to represent specific levels of safety.

LOSS-I - Indicates low potential for crash reduction
LOSS-II - Indicates low to moderate potential for crash reduction
LOSS-III - Indicates moderate to high potential for crash reduction
LOSS-IV - Indicates high potential for crash reduction
LOSS boundaries are calibrated by computing the $20^{\text {th }}$ and the $80^{\text {th }}$ percentiles using the Gamma Distribution Probability Density Function. Gradual change in the degree of deviation of the LOSS boundary line from the fitted model mean reflects the observed increase of variability in crashes as ADT increases. LOSS reflects how a segment of roadway or intersection is performing in regard to its expected crash frequency at a specific level of ADT.

SPF plots for both total crashes (see Figure 1) and for fatal and injury crashes (see Figure 2) also reflect this improvement in the crash record. LOSS improved within the LOSS II range for total crashes in the after period, and Injury/Fatal crashes improved to LOSS I in the after period (see Table 2), due to the decrease in both types of severe crashes. However, it is difficult to conclude that the overall decrease in some types of crashes can be attributed solely to the installation of the cable rail on the curves. However, as will be discussed in the following section, the cable rail has significantly reduced certain types of crashes.

Figure 1 - SPF for Total Crashes
I-25 (MP 102.50 - MP 107.50)
Before: 2002 to 2006 After: 7/2008 to 6/2013


Note: Safety Performance Function (SPF) Model - Urban Flat Rolling Mountainous 4-Lane Divided Freeway (Revised)

Figure 2 - SPF for Injury and Fatal Crashes I-25 (MP 102.50 - MP 107.50)
Before: 2002 to 2006 After: 7/2008 to 6/2013


Note: Safety Performance Function (SPF) Model rban Flat Rolling Mountainous 4-Lane Divided Freeway (Revised)

Table 2 - Safety Performance Function (SPF)

| I-25 MP 102.5 - 107.5 | Before | After | No Build After |  |
| :--- | :---: | :---: | :---: | :---: |
| EB Correction: | Yes | No | Yes |  |
| SPF Graph | Urban, 4-lane <br> Freeway | Urban, 4-lane <br> Freeway | Urban, 4-lane <br> Freeway |  |
| Total Crashes: |  |  |  |  |
| LOSS | LOSS II | LOSS II | LOSS II |  |
| CPMPY | 6.59 | 6.09 | 6.89 |  |
| Mean CPMPY | 8.08 | 8.43 | 8.43 |  |
| Proportion of Mean | 0.816 | 0.722 | 0.816 |  |
| Fatal \& Injury Crashes: |  |  |  |  |
| LOSS | LOSS II | LOSS I | LOSS II |  |
| CPMPY | 2.22 | 1.82 | 2.30 |  |
| Mean CPMPY | 3.17 | 3.29 | 3.29 |  |
| Proportion of Mean | 0.700 | 0.553 | 0.700 |  |

A more detailed review of the before and after crash record reveals that improvement in safety can be attributed to the installation of the cable rail. Table 3 shows a comparison of types of crashes that are most directly affected by the cable rail: overturning, head-on, and cable rail. The No Build After crashes were estimated using the increase in the mean of the SPF reflecting AADT in the after period for total and injury crashes found in Table 2 (increase is $1.04=$ $8.43 / 8.08$ ). Figures 1 and 2 also show that the number of crashes after construction was much better than it could have been without the project.

Table 3 - Results of Off-Road Left and Off-Median Crash Analyses

| I-25 MP 102.5-107.5 | Before | After | No Build After |
| :---: | :---: | :---: | :---: |
| Time Period: | $\begin{gathered} \text { 1/1/2002 to } \\ 12 / 31 / 2006 \text { (5 yr.) } \end{gathered}$ | $\begin{gathered} \text { 7/1/2008 to } \\ \text { 6/30/2013 (5 yr.) } \end{gathered}$ | $\begin{gathered} \text { 7/1/2008 to } \\ \text { 6/30/2013 (5 yr.) } \end{gathered}$ |
| Crash Types: (\#) |  |  |  |
| Overturning - Total | 15 | 7 | 16 |
| Fatal (fatalities) | 1 (1) | 1 (1) | 1 (1) |
| Injury (injuries) | 13 (21) | 5 (7) | 14 (22) |
| PDO | 1 | 1 | 1 |
| \% Reduction in Total |  | 56\% |  |
| Head-On - Total | 2 | 0 | 2 |
| Fatal (fatalities) | 1 (1) | 0 | 1 (1) |
| Injury (injuries) | 0 | 0 | 0 |
| PDO | 1 | 0 | 1 |
| \% Reduction in Total |  | 100\% |  |
| Cable Rail - Total | n/a | 21 | n/a |
| Injury (injuries) | n/a | 4 (6) | n/a |
| PDO | n/a | 17 | n/a |

Vision Zero Suite (VZS) includes benefit/cost (B/C) analyses within its procedures. The results of the B/C analysis are shown in Exhibit 3 for overturning, head-on, and cable rail crashes. The increase in cable rail crashes was factored into the analysis by increasing the cost of construction for the overturning and head-on crashes. During the 5 -year after period, there was 17 PDO cable rail crashes and 6 injuries. Over the design life of 20 years for the cable rail system, the increased cost of crashes would be $\$ 2,569,200$. Cable rail causes new crashes since it creates a barrier near the roadway, but the result is a trade-off of less serious crashes (PDOs primarily) instead of more serious fatal and injury crashes.

Exhibit 3 provides the Benefit/Cost calculations. The B/C ratio for the project is 2.26, showing that the improvement was justified.

Exhibit 3 - Benefit Cost Analysis - Overturning, Head-On, and Cable Rail Crashes Only



## ADT: 28,989 Length: 5.08



## ADT: 28,989 Length: 5.08



## ADT: 30,109 Length: 4.96



## ADT: 30,109 Length: 4.96

## Project Information

Project Name: Guard rail SH 165 north of Rye
Project Description: Install guard rail along SH 165 at select locations

CDOT Region: 2
Location: SH 165
Schedule:

## Project Def: 15771

Mile Points: 18.65 - 23.90
Work Start Date: 6/4/2007

## County: Custer/Pueblo

Length: 5.25 miles

Problem Description: As described in the Highway Safety Improvement Program (HSIP) application for this project, the five-year crash history showed a higher than expected number of off-road type crashes. These crashes were occurring on the curves of the roadways. The roadway has tight, compound, and reverse curves. Outside curves with steep drop-off exist along the roadway. The San Isabel Lake dam is located within this stretch of roadway. Driving across the dam, a very steep embankment exists on one side and the lake on the other.

Improvement Description: In 2007, guard rail was installed on the outside of select curves for drop-off protection and along both sides of the roadway across the dam. The cost of construction was $\$ 452,429$.

The HSIP application anticipated that off-road crash types would be impacted by this improvement. The following reductions in crashes were anticipated: fatal crashes - 60\%, injury crashes $-40 \%$, and property damage only $-0 \%$. The initial benefit/cost ratio was estimated to be 4.97 .

## Summary and Findings

The analysis of safety before and after the guard rail was installed along SH 165 showed significant safety improvements. For this segment of highway, there were 20 total crashes during the five-year period before the rail was installed (2002-2006). In the five years after construction (2008-2012), the number of crashes was decreased to 5. In addition, the number of injury and fatal crashes also diminished.

The guard rail improvement was directly responsible for decreases in the number and severity of off-road crashes. During the before period, there was one fatal tree crash and a number of injury crashes. The after period experienced no fatal crashes and a reduction in the number of injury crashes. Guard rail causes new crashes since it creates a barrier near the roadway, but the result is a trade-off of less serious crashes (PDOs primarily) instead of more serious fatal and injury crashes.

The ratio of benefits and cost for this project shows that benefits outweigh costs by a ratio of 12.67 to one. The result is an improvement that was certainly justified.

## Results of Safety Analyses

Using VZS, the review of before and after crash records shows a large decrease in the number of crashes; the total number of crashes decreased from 20 during the five-year period (2002 to 2006) before the guard rail was installed (see Table 1 and Exhibit 1) to 5 during the five-year after period (2008 to 2012) (see Table 1 and Exhibit 2). The number of serious crashes also showed a significant decrease:

- Before (2002-2006) - 1 fatal crashes with 1 fatalities (tree) and 11 injury crashes with 13 injuries
- After (2008-2012) - no fatal crashes and 3 injury crashes with 3 injuries

This decrease in severe crashes occurred along with a small decrease in traffic volumes on SH 165: 1,000 vehicles per day (vpd) estimated for the before period and 860 vpd in the after period. This combination of decreased traffic and decreased number of crashes also resulted in a decrease in the accident rates:

- Before (2002 - 2006): 11.04 crashes per million vehicle miles of travel (cpmvmt)
- After (2008-2012): 3.17 (cpmvmt)

Table 1 - Results of Overall Crash Analyses

| SH 165 MP 18.65-23.90 | Before | After |
| :---: | :---: | :---: |
| Time Period: | 1/1/2002 to 12/31/2006 (5 yr.) | 1/1/2008 to 12/31/2012 (5 yr.) |
| AADT | 993 vpd | 864 vpd |
| Filters: | Mainline Only/Non-Intersection | Mainline Only/Non-Intersection |
| Total Crashes | 20 | 5 |
| Fatal Crashes (Fatalities) | 1 (1) | 0 |
| Injury Crashes (Injuries) | 11 (13) | 3 (3) |
| Property Damage Only | 8 | 2 |
| Crash Types: \# (\%) [significance] |  |  |
| Fixed Objects | 15 (75.0\%) [99.81\%] | 3 (60.0\%) [100.00\%] |
| Rear End | 2 (10.0\%) | 0 |
| Overturning | 1 (5.0\%) | 2 (40.0\%) |
| Wild Animal | 1 (5.0\%) | 0 |
| Other Non-Collision | 1 (5.0\%) | 0 |
| Fixed Object Crashes: \# (\%) [significance] |  |  |
| Tree | 11 (73.3\%) [100.00\%] | 2 (66.7\%) |
| Embankment | 3 (20.0\%) | 0 |
| Delineator | 1 (6.7\%) | 0 |
| Guard rail | 0 | 1 (33.3\%) |

The magnitude of safety problems on select highway sections and intersections can be assessed through the use of Safety Performance Function (SPF) methodology. A SPF reflects the complex relationship between exposure (measured in ADT) and the crash count for a section of roadway measured in crashes per mile per year (CPMPY) or for an intersection,
measured in crashes per year. The SPF models provide an estimate for the expected crash frequency and severity for a range of ADT among similar facilities. This allows for an assessment of the magnitude of the safety problem from a frequency standpoint.

Development of the SPF lends itself well to the conceptual formulation of the Levels of Service of Safety (LOSS). The concept of level of service uses qualitative measures that characterize safety of a roadway segment in reference to its expected performance and severity. If the level of safety predicted by the SPF represents a normal or expected number of crashes at a specific level of ADT, then the degree of deviation from the normal can be stratified to represent specific levels of safety.

LOSS-I - Indicates low potential for crash reduction
LOSS-II - Indicates low to moderate potential for crash reduction
LOSS-III - Indicates moderate to high potential for crash reduction
LOSS-IV - Indicates high potential for crash reduction
LOSS boundaries are calibrated by computing the $20^{\text {th }}$ and the $80^{\text {th }}$ percentiles using the Gamma Distribution Probability Density Function. Gradual change in the degree of deviation of the LOSS boundary line from the fitted model mean reflects the observed increase of variability in crashes as ADT increases. LOSS reflects how a segment of roadway or intersection is performing in regard to its expected crash frequency at a specific level of ADT.

SPF plots for both total crashes (see Figure 1) and for fatal and injury crashes (see Figure 2) also reflect this improvement in the crash record. LOSS improved to LOSS I from LOSS III for total crashes in the after period, and Injury/Fatal crashes improved to LOSS II in the after period (see Table 2), due to the decrease in both types of severe crashes. However, it is difficult to conclude that the overall decrease in some types of crashes can be attributed solely to the installation of the guard rail on the curves. However, as will be discussed in the following section, the guard rail has significantly reduced certain types of crashes.

Figure 1 - SPF for Total Crashes
SH 165 (MP 18.65 - MP 23.90)
Before: 2002 to 2006 After: 2008 to 2012


Note: Safety Performance Function (SPF) Model - Rural Mountainous 2-Lane Undivided Highway

Figure 2 - SPF for Injury and Fatal Crashes
SH 165 (MP 18.65 - MP 23.90)
Before: 2002 to 2006 After: 2008 to 2012


Note: Safety Performance Function (SPF) Model - Rural Mountainous 2-Lane Undivided Highway

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ULLEVIG

Table 2 - Safety Performance Function (SPF)

| SH 165 MP 18.65-23.90 | Before | After | No Build After |  |
| :--- | :---: | :---: | :---: | :---: |
| EB Correction: | Yes | No | Yes |  |
| SPF Graph | Rural, Mountainous, <br> 2-Lane, Undivided <br> Highway | Rural, Mountainous, <br> 2-Lane, Undivided <br> Highway | Rural, Mountainous, <br> 2-Lane, Undivided <br> Highway |  |
| Total Crashes: |  |  |  |  |
| LOSS | LOSS III | LOSS I | LOSS III |  |
| CPMPY | 0.65 | 0.19 | 0.58 |  |
| Mean CPMPY | 0.56 | 0.50 | 0.50 |  |
| Proportion of Mean | 1.16 | 0.90 | 1.16 |  |
| Fatal \& Injury Crashes: |  |  |  |  |
| LOSS | LOSS III | LOSS II | LOSS III |  |
| CPMPY | 0.30 | 0.11 | 0.27 |  |
| Mean CPMPY | 0.23 | 0.21 | 0.21 |  |
| Proportion of Mean | 1.30 | 0.52 | 1.30 |  |

A more detailed review of the before and after crash record reveals that a significant improvement in safety can be attributed to the installation of the guard rail. Table 3 shows a comparison of type of crashes that are most directly affected by the guard rail: off-road right. The No Build After crashes were estimated using the decrease in the mean predicted by the SPF reflecting AADT for the after period, of the SPF for total and injury crashes found in Table 2 (decrease is $0.89=0.50 / 0.56$ ). Figures 1 and $\mathbf{2}$ also show that the number of crashes after construction was much better than it could have been without the project.

Table 3 - Results of Off-Road Left and Off-Median Crash Analyses

|  | Before | After | No Build After |
| :---: | :---: | :---: | :---: |
| Time Period: | $1 / 1 / 2002$ to | $1 / 1 / 2008$ to | $1 / 1 / 2008$ to |
|  | $12 / 31 / 2006(5 \mathrm{yr})$. | $12 / 31 / 2012(5 \mathrm{yr})$. | $12 / 31 / 2012(5 \mathrm{yr})$. |
| Crash Types: | $\mathbf{y y y}$ |  |  |
| Off-Road RIght - Total | 10 | $\mathbf{2}$ | $\mathbf{9}$ |
| Fatal (fatalities) | $7(9)$ | 0 | $1(1)$ |
| Injury (injuries) | 2 | $1(1)$ | $6(8)$ |
| PDO | n/a | 1 | 2 |
| \% Reduction in Total | n/a | $78 \%$ | n/a |
| Guard Rail - Total | n/a | $\mathbf{1}$ | $\mathrm{n} / \mathrm{a}$ |
| Injury (injuries) |  | 0 | $\mathrm{n} / \mathrm{a}$ |
| PDO | 1 |  |  |

Vision Zero Suite (VZS) includes benefit/cost (B/C) analyses within its procedures. The results of the B/C analysis are shown in Exhibit 3 for off-road right crashes. The increase in guard rail crashes was factored into the analysis by increasing the cost of construction for the off-road right crashes. During the 5 -year after period, there was 1 PDO guard rail crashes. Over the design life of 20 years for the cable rail system, the increased cost of crashes would be
$\$ 37,200$. Guard rail causes new crashes since it creates a barrier near the roadway, but the result is a trade-off of less serious crashes (PDOs primarily) instead of more serious fatal and injury crashes.

Exhibit 3 provides the Benefit/Cost calculations. The B/C ratio for the project is 12.67, showing that the improvement was certainly justified.

Exhibit 3 - Benefit Cost Analysis - Off-Road Right Crashes Only



## ADT: 993 Length: 5.34



## ADT: 993 Length: 5.34



## ADT: 864 Length: 5.28



## ADT: 864 Length: 5.28

## Project Information

Project Name: $\quad$ SH $93-$ SH 72 (MP 7.57) to SH 128 (MP 11.78)
Project Description: Upgrade Signal at SH 72 / SH 93 and Install Weather-Related Road Closure System

CDOT Region: 1
Location: SH 93

Project Def: 15828
Mile Points: from: 7.47 to 11.83
Work Start Date: 3/5/2007

County: Jefferson
Length: 4.21 miles
Completion Date: 12/10/2007

Problem Description: Two problems were identified at the intersection of SH 72/SH 93 and along SH 93 to the north for approximately four miles to the intersection with SH 128. As described in the Highway Safety Improvement Program (HSIP) application, the five-year (1999 - 2003) crash history at the intersection of SH 72/SH 93 (which had span wire signalization) showed 40 crashes occurred including 15 approach turn, 9 rear-end, and 7 broadside crashes. These involved 1 fatal and 15 injury crashes. The section of SH 93 across Rocky Flats is subject to periodic sustained high winds and wind gusts in excess of 80 mph . High winds and gusts contribute to high-profile vehicles overturning and reduced visibility due to "white-out" and "ground blizzard" conditions which may contribute to the loss of vehicle control. In a three-year period (2001 - 2003), there were 29 PDO and 8 injury crashes.

Improvement Description: In 2007, new mast arms, signal heads with LED lenses, backplates, and a new detection system was constructed at the intersection. New pavement markings were placed to delineate lanes, cross walks and stop lines. The channelizing islands and curb ramps were also improved. To address the adverse wind and weather conditions across Rocky Flats, a variable message sign (VMS) was installed on northbound SH 93 just south of $64{ }^{\text {th }}$ Avenue (MP 3.794) to provide notice of closures and/or restrictions. Road closure gates were installed on the north leg of the SH 72/SH 93 intersection to facilitate temporary weather-related road closures. The cost of construction for the signal installation and related intersection improvements was approximately $\$ 400,000$ and $\$ 199,548$ for the weather information/closure installations. The total cost for the combined project was $\$ 599,548$.

The HSIP application anticipated that three crash types could potentially be reduced by $15 \%$ by the signal improvement: approach turn, broadside, and rear-end crashes. The VMS and closure gate project was estimate to reduce weather-related crashes by $25 \%$. The initial benefit/cost ratios were estimated to be 1.72 for the signal and intersection improvements and 1.17 for the weather-related infrastructure.

## Summary and Findings

The analysis of safety before and after construction showed significant safety improvements resulted from the weather-related infrastructure that was installed along SH 93 (between SH 72 and SH 128) and the signal and related improvements at the SH 72 / SH 93 intersection. For this segment of SH 93, there were 260 total crashes during the five-year period before construction (2002-2006). In the five years after construction (2008-2012), the number of crashes decreased to 189. Although daily volumes decrease throughout the study period, the
non-intersection crash rate on SH 93 also experienced a reduction. In addition, the number of injury and fatal crashes also diminished significantly.

The weather-related infrastructure (variable message sign and road closure gates) resulted in decreases in the number and severity of wind caused crashes. This portion of the project resulted in a Benefit/Cost ratio of 1.42. The signal and related improvements at the SH 72/SH 93 intersection resulted in a large increase in safety. During the before period, there was one fatal approach turn collision and a number of injury crashes (broadside, rear-end, sideswipesame, and approach turn crashes). The after period experienced no fatal crashes and a reduction in the number of injury crashes. This portion of the project resulted in a significant Benefit/Cost ratio of 14.93. In summary, the ratio of benefits and cost for this project shows that benefits outweigh costs by a ratio of 16.35 to one. The result is improvements that were certainly justified.

## Results of Safety Analyses

Using VZS, the review of before and after crash records shows a decrease in the number of crashes; the total number of crashes decreased from 260 during the five-year period ( 2002 to 2006) before the project was constructed (see Table 1 and Exhibit 1) to 189 during the fiveyear after period (2008 to 2012) (see Table 1 and Exhibit 2). The number of serious crashes showed a more significant decrease:

- Before (2002-2006) - 5 fatal crashes with 5 fatalities ( 2 head-on and 1 each involving overturning, approach turn, and a fixed object) and 75 injury crashes with 110 injuries
- After (2008-2012) - 2 fatal crashes (both were sideswipe opposite) and 45 injury crashes with 73 injuries

This decrease in severe crashes occurred along with a decrease in traffic volumes on SH 93: 18,300 vehicles per day (vpd) estimated for the before period and 15,900 vpd in the after period. The Rocky Flats Plant was a large traffic generator along SH 93. It produced weapons-grade plutonium through the mid-1990's. Clean-up of the site was completed in 2005, and it became a National Wildlife Refuge in 2007.

Because this project involved two separate improvements, the crash record in Table 1 has separate listings for the non-intersection crashes along SH 93 and the intersection-related crashes at the intersection of SH 72 and SH 93 . The crash rate along SH 93 showed a decrease in the accident rates as the proportional decrease in the number of crashes was greater than the decrease in traffic:

- Before (2002 - 2006): 1.13 crashes per million vehicle miles of travel (cpmvmt)
- After (2008-2012): 1.03 (cpmvmt)

Table 1 also shows that both the number and severity of crashes at the SH 72/SH 93 intersection also improved in the after period.

Table 1 - Results of Overall Crash Analyses

|  | Before | After |
| :---: | :---: | :---: |
| Time Period: | 1/1/2002 to 12/31/2006 (5 yr.) | 1/1/2008 to 12/31/2012 (5 yr.) |
| AADT | 18,300 vpd | 15,900 vpd |
| Filters: | None - All Crashes Included | None - All Crashes Included |
| Total Crashes | 260 | 189 |
| Fatal Crashes (Fatalities) | 5 (5) | 2 (3) |
| Injury Crashes (Injuries) | 75 (110) | 45 (73) |
| Property Damage Only | 180 | 142 |
| SH 93 (MP 7.47 to MP 11.83) - Non-intersection / Mainline only |  |  |
| Total Crashes | 173 | 135 |
| Fatal Crashes (Fatalities) | 4 (4) | 2 (3) |
| Injury Crashes (Injuries) | 46 (64) | 33 (57) |
| Property Damage Only | 123 | 100 |
| Crash Types: \# (\%) [significance] |  |  |
| Fixed Objects | 45 (26.0\%) [99.42\%] | 43 (31.9\%) [99.94\%] |
| Rear End | 37 (21.4\%) | 25 (18.5\%) |
| Overturning | 24 (13.9\%) | 13 (9.6\%) |
| Wild Animal | 19 (11.0\%) | 24 (17.8\%) |
| Head-On | 13 (7.5\%) | 11 (8.1\%) |
| Sideswipe Opposite | 11 (6.4\%) | 8 (5.9\%) [99.57\%] |
| Sideswipe Same | 9 (5.2\%) | 2 (1.4\%) |
| Fixed Object Crashes: \# (\% of FO) [significance] |  |  |
| Fence | 22 (48.9\%) [100.0\%] | 15 (37.4\%) [99.80\%] |
| Guard Rail | 1 (1.9\%) | 6 (14.0\%) [100.00\%] |
| Adverse Weather Conditions: \# (\% of non-intersection) [significance] |  |  |
| Snow/Sleet/Hail | 39 (22.5\%) [100.0\%] | 35 (25.9\%) [100\%] |
| Wind | 22 (12.7\%) [100.0\%] | 10 (7.4\%) [100\%] |
| Adverse Road Conditions: \# (\% of non-intersection) [significance] |  |  |
| Snowy | 20 (11.6\%) [100.0\%] | 13(9.6\%) [100\%] |
| Icy | 25 (14.5\%) | 29 (21.5\%) [100\%] |
| Slushy | 10 (5.8\%) [100.0\%] | 3 (2.2\%) |
| SH 72 (MP 10.6 to 10.70) / SH 93 (MP 7.47-7.67) - Intersection Crashes only |  |  |
| Total Crashes | 43 | 32 |
| Fatal Crashes (Fatalities) | 1 (1) | 0 |
| Injury Crashes (Injuries) | 11 (19) | 7 (10) |
| Property Damage Only | 31 | 25 |
| Crash Types: \# (\%) [significance] |  |  |
| Rear End | 19 (44.2\%) | 24 (75.0\%) |
| Broadside | 11 (25.6\%) | 4 (12.5\%) |
| Approach Turn | 6 (14.0\%) | 2 (6.2\%) |
| Sideswipe Same | 3 (7.0\%) | 0 |

The magnitude of safety problems on select highway sections and intersections can be assessed through the use of Safety Performance Function (SPF) methodology. A SPF reflects the complex relationship between exposure (measured in ADT) and the crash count for a section of roadway measured in crashes per mile per year (CPMPY) or for an intersection, measured in crashes per year. The SPF models provide an estimate for the expected crash frequency and severity for a range of ADT among similar facilities. This allows for an assessment of the magnitude of the safety problem from a frequency standpoint.

Development of the SPF lends itself well to the conceptual formulation of the Levels of Service of Safety (LOSS). The concept of level of service uses qualitative measures that characterize safety of a roadway segment in reference to its expected performance and severity. If the level of safety predicted by the SPF represents a normal or expected number of crashes at a specific level of ADT, then the degree of deviation from the normal can be stratified to represent specific levels of safety.

LOSS-I - Indicates low potential for crash reduction
LOSS-II - Indicates low to moderate potential for crash reduction
LOSS-III - Indicates moderate to high potential for crash reduction
LOSS-IV - Indicates high potential for crash reduction
LOSS boundaries are calibrated by computing the $20^{\text {th }}$ and the $80^{\text {th }}$ percentiles using the Gamma Distribution Probability Density Function. Gradual change in the degree of deviation of the LOSS boundary line from the fitted model mean reflects the observed increase of variability in crashes as ADT increases. LOSS reflects how a segment of roadway or intersection is performing in regard to its expected crash frequency at a specific level of ADT.

SPF analyses have not been preformed for urban (or rural) 2-lane, divided, signalized, 4-leg intersections so comparisons are not available for the intersection of SH 72/SH 93. SPF plots are available for rural, flat and rolling, 2-lane, undivided highways for both total crashes (see Figure 1) and for fatal and injury crashes (see Figure 2). LOSS improved within the LOSS IV range for total crashes in the after period, and Injury/Fatal crashes improved within the LOSS III range in the after period (see Table 2), due to the decrease in both types of severe crashes.

Figure 1 - SPF for Total Crashes
SH 93 (MP 7.47 to MP 11.83)
Before: $\mathbf{2 0 0 2}$ to 2006 After: 2008 to 2012


Note: Safety Performance Function (SPF) Model: Colorado - Rural Flat and Rolling 2-Lane Undivided Highway

Figure 2 - SPF for Injury and Fatal Crashes
SH 93 (MP 7.47 to MP 11.83)
Before: 2002 to 2006 After: 2008 to 2012


Note: Safety Performance Function (SPF) Model: Colorado - Rural Flat and Rolling 2-Lane Undivided Highway

Table 2 - SH 93 (MP 7.47 to MP 11.83) - Safety Performance Function (SPF)

|  | Before | After | No Build After |  |
| :--- | :---: | :---: | :---: | :---: |
| EB Correction: | Yes | No | Yes |  |
| SPF Graph | Rural Flat \& Rolling, <br> 2-lane Undivided <br> Highway |  <br> Rolling, 2-lane <br> Undivided Highway | Rural Flat \& Rolling, <br> 2-lane Undivided <br> Highway |  |
| Total Crashes: | LOSS IV | LOSS IV | LOSS IV |  |
| LOSS | 7.10 | 5.96 | 6.69 |  |
| CPMPY | 3.43 | 3.23 | 3.23 |  |
| Mean CPMPY | 2.070 | 1.845 | 2.070 |  |
| Proportion of Mean |  |  |  |  |
| Fatal \& Injury Crashes: | LOSS III | LOSS III | LOSS III |  |
| LOSS | 2.01 | 1.54 | 1.87 |  |
| CPMPY | 1.57 | 1.46 | 1.46 |  |
| Mean CPMPY | 1.280 | 1.055 | 1.280 |  |
| Proportion of Mean |  |  |  |  |

A more detailed review of the before and after crash records reveals that a significant improvements in safety can be attributed to the installation of the weather information/closure system and the signal system upgrade. Table 3 shows that wind-related crashes were reduced in the after period. The No Build After crashes were estimated using the decrease in the median of the SPF for total crashes found in Table 2 (decrease is $0.942=6.69 / 7.10$ ). Table 4 shows the decrease in broadside, rear-end, sideswipe (same), and approach turn crashes was significant. The No Build After crashes were estimated using the decrease in the total approach volumes to the intersection between the before and after periods (decrease is $0.916=41,540$ / 45,355 ). Particularly notable is the improvement in both the number and severity of approach turn crashes due to allowing protected-only left turns in the after period.

Vision Zero Suite (VZS) includes benefit/cost (B/C) analyses within its procedures. The results of the B/C analysis are shown in Exhibit 3 for wind-related crashes on SH 93 between SH 72 and SH 128 and Exhibit 4 for broadside, rear-end, sideswipe (same), and approach turn crashes at the SH 72/SH 93 intersection. The B/C ratio for wind-related crashes on SH 93 is 1.42 and the B/C ratio for crashes at the SH 72/SH 93 intersection is 14.93. The resulting B/C ratio for the combined project is 16.35 ( $1.42+14.93$ ), showing that these improvements was certainly justified, by themselves and together.

Table 3 - Results of SH 93 (MP 7.47 to MP 11.83) Wind-related Crash Analyses

|  | Before | After | No Build After |
| :--- | :---: | :---: | :---: |
| Time Period: | $1 / 1 / 2002$ to <br> $12 / 31 / 2006 ~(5 ~ y r) ~$. | $1 / 1 / 2008 ~ t o ~$ <br> $12 / 31 / 2012 ~(5 ~ y r) ~$. | $1 / 1 / 2008$ to <br> $12 / 31 / 2012(5 \mathrm{yr}$.) |
| Crash Types: | $\mathbf{2 2}$ | $\mathbf{1 0}$ | $\mathbf{2 1}$ |
| Wind-related - Total | $3(3)$ | $2(2)$ | $3(3)$ |
| Injury (injuries) | 19 | 8 | 18 |
| PDO |  | $33 \% / 56 \%$ |  |
| \% Reduction in Total - <br> (INJ / PDO) |  |  |  |

Table 4 - Results of Improved Signalization Crash Analyses at SH 72 I SH 93 Intersection

|  | Before | After | No Build After |
| :---: | :---: | :---: | :---: |
| Time Period: | $\begin{gathered} \hline 1 / 1 / 2002 \text { to } \\ 12 / 31 / 2006 \text { (5 yr.) } \end{gathered}$ | $\begin{gathered} \text { 1/1/2008 to } \\ 12 / 31 / 2012 \text { (5 yr.) } \end{gathered}$ | $\begin{gathered} \hline 1 / 1 / 2008 \text { to } \\ 12 / 31 / 2012 \text { (5 yr.) } \end{gathered}$ |
| Crash Types: |  |  |  |
| Broadside - Total | 11 | 4 | 10 |
| Injury (injuries) | 3 (5) | 2 (3) | 3 (5) |
| PDO | 8 | 2 | 7 |
| Read End - Total | 19 | 24 | 17 |
| Injury (injuries) | 5 (5) | 4 (6) | 4 (4) |
| PDO | 14 | 20 | 13 |
| Sideswipe (Same) - Total | 3 | 0 | 3 |
| Injury (injuries) | 0 | 0 | 0 |
| PDO | 3 | 0 | 3 |
| Approach Turn Total | 6 | 2 | 6 |
| Fatal (fatalities) | 1 (1) | 0 | 1 (1) |
| Injury (injuries) | 1 (6) | 0 (0) | 1 (6) |
| PDO | 4 | 2 | 4 |
| Total | 39 | 30 | 36 |
| Fatal (fatalities) | 1 (1) | 0 | 1 (1) |
| Injury (injuries) | 9 (16) | 6 (9) | 8(15) |
| PDO | 29 | 24 | 27 |
| \% Reduction in Total (FAT / INJ / PDO) |  | 100\% / 40\% / 11\% |  |

## Exhibit 3 - Benefit Cost Analysis - SH 93 (MP 7.47 to MP 11.83Non-intersection Windrelated Crashes Only



Exhibit 4 - Benefit Cost Analysis - Broadside, Rear-end, Sideswipe (same) and Approach Turn Crashes at Intersection of SH 72 (MP 10.6 to 10.70) and SH 93 (MP 7.47 - 7.67)



## ADT: 18,326 Length: 4.57



## ADT: 18,326 Length: 4.57



## ADT: 15,901 Length: 4.53



## ADT: 15,901 Length: 4.53

## Project Information

Project Name: $\quad$ SH $52 / 95^{\text {th }}$ Street
Project Description: Intersection Improvements
CDOT Region:
Location: SH 52

Project Def: 15861
Mile Points: 3.16
Work Start Date: approx. 8/2007

County: Boulder
Length: N/A
Completion Date: approx. 2/2008

Problem Description: As described in the Highway Safety Improvement Program (HSIP) application for this project, the five-year crash history (1998-2002) showed a number of broadside and approach turn crashes.

Improvement Description: Between late 2007 and early 2008, the northbound and southbound pavement was widened. Additionally, northbound and southbound left-turn lanes were constructed and protected phasing was added. The cost of construction was $\$ 150,000$.

The HSIP application anticipated that the following reductions in crashes might be realized by the improvement anticipated: northbound/southbound crashes - 35\%. The initial benefit/cost ratio was estimated to be 2.52 .

## Summary and Findings

The analysis of safety before and after the intersection at SH 52 and $95^{\text {th }}$ Street was reconstructed showed some reduction in the types of crashes that left-turn lane channelization and left-turn phasing is intended to mitigate. For this intersection, there were 19 total crashes (at intersection, intersection related) during the five-year period before the left turn lanes and signal phasing was installed (2002-2006). In the five years after construction (2009-2013), the number of crashes decreased slightly to 17. In addition, the number of injury crashes also diminished.

The northbound/southbound channelization and phasing improvements were directly responsible for decrease in the northbound and southbound crashes. During the before period, there was six northbound/southbound injury crashes and six property damage only crashes. The after period experienced two injury crashes of this type and two property damage only crashes.

The ratio of benefits and cost for this project shows that benefits outweigh costs by a ratio of 13.37 to one. The result is an improvement that was justified.

## Results of Safety Analyses

Using Vision Zero Suite, the review of before and after crash records shows a slight decrease in the number of crashes; the total number of crashes decreased from 19 during the five-year period (2002 to 2006) before the intersection was reconstructed (see Table 1 and Exhibit 1) to 17 during the five-year after period (2009 to 2013) (see Table 1 and Exhibit 2). The number of serious crashes showed a more significant decrease:

- Before (2002 - 2006) - 9 injury crashes with 16 injuries
- After (2009-2013) - 6 injury crashes with 7 injuries
- Reduction occurred predominantly in the N-S direction where improvements have been constructed.

Along with the decrease in severe crashes there was also a modest decrease in traffic volumes on SH 52A: 9,800 vehicles per day (vpd) estimated for the before period and 9,400 vpd in the after period. Even though there was a small reduction in traffic volume a small decrease was observed in crash rate in the after period.

- Before (2002 - 2006): 0.65 crashes per million entering vehicles (cpmev)
- After (2009 - 2013): 0.60 (cpmev)

Table 1 - Results of Overall Crash Analyses

|  | Before | After |
| :--- | :---: | :---: |
| Time Period: | $1 / 1 / 2002$ to $12 / 31 / 2006$ (5 yr.) | $1 / 1 / 2009$ to $12 / 31 / 2013$ (5 yr.) |
| AADT | 9,842 vpd / 6,200 vpd | 9,351 vpd / 6,200 vpd |
| Filters: | At Intersection <br> Intersection Related | At Intersection <br> Intersection Related |
| Total Crashes | 19 | 17 |
| Fatal Crashes (Fatalities) | 0 | 0 |
| Injury Crashes (Injuries) | $9(16)$ | $6(7)$ |
| Property Damage Only | 10 | 11 |
| Crash Types: \# (\%) [significance] |  | $8(47.1 \%)$ |
| Rear End | $8(42.1 \%)$ | $5(29.4 \%)[96.04 \%]$ |
| Approach Turn | $7(36.8 \%)[99.44 \%]$ | $2(11.8 \%)$ |
| Broadside | $2(10.5 \%)$ |  |

Normally, the magnitude of safety problems on highway sections and intersections can be assessed through the use of Safety Performance Function (SPF) methodology. However in the case the intersection of SH 52A with $95^{\text {th }}$ Avenue, no SPF for this intersection configuration has been developed.

A more detailed review of the before and after crash record reveals that notable improvement in safety in the N/S direction can be attributed to the channelization and introduction of the left turn phase the northbound and southbound left-turns. Table 2 shows a comparison of the crash type that is most directly affected by the intersection improvements: northbound/southbound vehicle crashes. The No Build After crashes were estimated using the change in the average daily traffic volumes found in Table 1.

Table 2 - Results of Northbound/Southbound Protected Left-Turns Crash Analyses

|  | Before | After | No Build After |
| :---: | :---: | :---: | :---: |
| Time Period: | $1 / 1 / 2002$ to | $1 / 1 / 2009$ to | $1 / 1 / 2009$ to |
|  | $12 / 31 / 2006$ (5 yr.) | $12 / 31 / 2013(5 \mathrm{yr}$. $)$ | $12 / 31 / 2013$ (5 yr.) |
| Crash Types: | $\mathbf{y y y}$ |  |  |
| NBISB Crashes - Total | 0 | $\mathbf{4}$ | $\mathbf{1 2}$ |
| Fatal (fatalities) | $6(10)$ | 0 | 0 |
| Injury (injuries) | 6 | $2(2)$ | $6(10)$ |
| PDO |  | 2 | 6 |
| \% Reduction in Total |  | $66 \%$ |  |

Vision Zero Suite includes benefit/cost (B/C) analyses within its procedures. The results of the $B / C$ analysis are shown in Exhibit 3 for the northbound/southbound crashes. The B/C ratio for decreasing the approach turn crashes is 13.37 , showing that the improvement was justified.

Exhibit 3 - Benefit Cost Analysis - Northbound/Southbound Crashes



## ADT: 9,842 Length: 0.10




## ADT: 9,351 Length: 0.09



## ADT: 9,351 Length: 0.09

## Project Information

Project Name: US $34 / 11^{\text {th }}$ Avenue
Project Description: Intersection Improvements
CDOT Region: 4
Location: SH 34
Project Def: 15862
Mile Points: 112.23
Schedule:
Work Start Date: approx. 9/2007

## County: Weld

Length: N/A
Completion Date: approx. 4/2008

Problem Description: As described in the Highway Safety Improvement Program (HSIP) application for this project, the five-year crash history showed a higher than expected number of rear-end type crashes. The traffic signal is over 20 years old with 8 " yellow and green signal heads and no back plates, which does not meet CDOT standards.

Improvement Description: In late 2007 and early 2008, the traffic signal was upgraded to 12" LED signal heads with back-plates. Additionally, signal heads were installed on new poles and mast arms. Video detection was installed and the street lighting was upgraded. The cost of construction was $\$ 462,403$.

The HSIP application anticipated that four crash types would be impacted by this improvement: rear-end, approach turn, broadside, and pedestrian type crashes. It was anticipated that there would be a $15 \%$ crash reduction for these crash types. The initial benefit/cost ratio was estimated to be 2.03.

## Summary and Findings

The analysis of safety before and after the signal was upgraded at US 34 and $11^{\text {th }}$ Avenue showed significant safety improvements. For this intersection, there were 106 total crashes during the five-year period before the upgrades (2001 - 2005). In the five years after construction (2009-2013), the number of crashes was decreased to 100 . Since daily volumes continued to increase throughout the study period, the crash rate was reduced. In addition, the number of injury and fatal crashes also diminished.

The signal upgrade was responsible for decreases in the number and severity of rear end, approach turn, broadside, and pedestrian crashes. During the before period, there was one fatal pedestrian crash and a number of injury crashes. The after period experienced no fatal crashes and a reduction in the number of injury crashes.

The ratio of benefits and cost for this project shows that benefits outweigh costs by a ratio of 9.69 to one. The result is an improvement that was justified.

## Results of Safety Analyses

Using VZS, the review of before and after crash records shows a decrease in the number of crashes; the total number of crashes decreased from 104 during the five-year period (2001 to 2005) before the signal was upgraded (see Table 1 and Exhibit 1) to 100 during the five-year after period (2009 to 2013) (see Table 1 and Exhibit 2). The number of serious crashes showed a more significant decrease:

- Before (2001 - 2005) - 1 fatal crash with 1 fatality (pedestrian) and 28 injury crashes with 41 injuries
- After (2009-2013) - no fatal crashes and 18 injury crashes with 26 injuries

This decrease in severe crashes occurred in spite of a modest increase in traffic volumes at the intersection. This combination of increased traffic and decreased number of crashes also resulted in a decrease in the accident rates.

Table 1 - Results of Overall Crash Analyses

|  | Before | After |
| :--- | :---: | :---: |
| Time Period: | $1 / 1 / 2001$ to $12 / 31 / 2005$ (5 yr.) | $1 / 1 / 2009$ to 12/31/2013 (5 yr.) |
| AADT (US 34/11 ${ }^{\text {th }}$ Ave) | $36,000 / 13,000 \mathrm{vpd}$ | $38,000 / 16,500 \mathrm{vpd}$ |
| Filters: | At Intersection <br> Intersection Related | At Intersection <br> Intersection Related |
| Total Crashes | 104 | 100 |
| Fatal Crashes (Fatalities) | $1(1)$ | 0 |
| Injury Crashes (Injuries) | $28(41)$ | $18(26)$ |
| Property Damage Only | 75 | 82 |
| Crash Types: \# (\%) [significance] |  | $54(54.0 \%)[96.84 \%]$ |
| Rear End | $42(40.4 \%)$ | $15(15.0 \%)$ |
| Approach Turn | $21(20.2 \%)$ | $12(12.0 \%)$ |
| Sideswipe Same | $15(14.4 \%)[99.37 \%]$ | $12(12.0 \%)$ |
| Broadside | $14(13.5 \%)$ | $6(6.0 \%)$ |
| Fixed Object | $3(2.9 \%)$ | $1(1.0 \%)$ |
| Pedestrian | $1(1.0 \%)$ |  |

The magnitude of safety problems on select highway sections and intersections can be assessed through the use of Safety Performance Function (SPF) methodology. A SPF reflects the complex relationship between exposure (measured in ADT) and the crash count for a section of roadway measured in crashes per mile per year (CPMPY) or for an intersection, measured in crashes per year. The SPF models provide an estimate for the expected crash frequency and severity for a range of ADT among similar facilities. This allows for an assessment of the magnitude of the safety problem from a frequency standpoint.

Development of the SPF lends itself well to the conceptual formulation of the Levels of Service of Safety (LOSS). The concept of level of service uses qualitative measures that characterize safety of a roadway segment in reference to its expected performance and severity. If the level of safety predicted by the SPF represents a normal or expected number of crashes at a specific
level of ADT, then the degree of deviation from the normal can be stratified to represent specific levels of safety.

LOSS-I - Indicates low potential for crash reduction
LOSS-II - Indicates low to moderate potential for crash reduction
LOSS-III - Indicates moderate to high potential for crash reduction
LOSS-IV - Indicates high potential for crash reduction
LOSS boundaries are calibrated by computing the $20^{\text {th }}$ and the $80^{\text {th }}$ percentiles using the Gamma Distribution Probability Density Function. Gradual change in the degree of deviation of the LOSS boundary line from the fitted model mean reflects the observed increase of variability in crashes as ADT increases. LOSS reflects how a segment of roadway or intersection is performing in regard to its expected crash frequency at a specific level of ADT.

SPF plots for both total crashes (see Figures 1 and 3 ) and for fatal and injury crashes (see Figure 2 and 4) also reflect this improvement in the crash record. LOSS improved to the LOSS II range for total crashes in the after period from LOSS III, and Injury/Fatal crashes improved to LOSS I in the after period (see Table 2), due to the decrease in both types of severe crashes.

A more detailed review of the before and after crash record reveals that a significant improvement in safety can be attributed to the upgrade of the signal. Table 3 shows a comparison of four types of crashes that are most directly affected by the improvement: rear end, approach turn, broadside, and pedestrian. The No Build After crashes were estimated using the increase in the median of the SPF for total crashes found in Table 2 (increase is 1.17 $=21.85 / 18.75$ ). Figures 3 and 4 also show that the number of crashes after construction was much better than it could have been without the project.

Figure 1 - SPF for Total Crashes - Before
US 34 / $11^{\text {th }}$ Avenue
2001 to 2005


Note: Safety Performance Function (SPF) Model - Urban 4-Lane Divided Signalized 4-Leg Intersection

Figure 2 - SPF for Injury and Fatal Crashes - Before US $34 / 11^{\text {th }}$ Avenue 2001 to 2005


Note: Safety Performance Function (SPF) Model - Urban 4-Lane Divided Signalized 4-Leg Intersection

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Figure 3 - SPF for Total Crashes - After
US 34 / $11^{\text {th }}$ Avenue
2009 to 2013


Note: Safety Performance Function (SPF) Model - Urban 4-Lane Divided Signalized 4-Leg Intersection

Figure 3 - SPF for Injury and Fatal Crashes - After US $34 / 11^{\text {th }}$ Avenue

2009 to 2013


Note: Safety Performance Function (SPF) Model - Urban 4-Lane Divided Signalized 4-Leg Intersection

Table 2 - Safety Performance Function (SPF)

|  | Before | After | No Build After |
| :--- | :---: | :---: | :---: |
| EB Correction: | Yes | No | Yes |
| SPF Graph | Urban, 4-lane, <br> Divided, Signalized, <br> 4-Leg Intersection | Urban, 4-lane, <br> Divided, Signalized, <br> 4-Leg Intersection | Urban, 4-lane, <br> Divided, Signalized, <br> 4-Leg Intersection |

Total Crashes:

| LOSS | LOSS III | LOSS II | LOSS III |
| :--- | :---: | :---: | :---: |
| CPY | 20.65 | 20.00 | 24.06 |
| Mean CPY | 18.75 | 21.85 | 21.85 |
| Proportion of Mean | 1.10 | 0.92 | 1.10 |

Fatal \& Injury Crashes:

| LOSS | LOSS III | LOSS I | LOSS III |
| :--- | :---: | :---: | :---: |
| CPY | 5.73 | 3.60 | 6.58 |
| Mean CPY | 5.43 | 6.21 | 6.21 |
| Proportion of Mean | 1.06 | 0.58 | 10.6 |

Vision Zero Suite (VZS) includes benefit/cost (B/C) analyses within its procedures. Because there was an increase in the number of PDO crashes, the cost of these crashes was added to the cost of construction in the analysis. The results of the B/C analysis are shown in Exhibit 3 for the impacted crash types. As shown, the B/C ratio for rear end, approach turn, broadside, and pedestrian crashes is 9.69 , showing that the improvement was justified.

Table 3 - Results of Crash Analyses

|  | Before | After | No Build After |
| :---: | :---: | :---: | :---: |
| Time Period: | $\begin{gathered} \hline 1 / 1 / 2001 \text { to } \\ 12 / 31 / 2005 \text { ( } 5 \text { yr.) } \end{gathered}$ | $\begin{gathered} \text { 1/1/2009 to } \\ 12 / 31 / 2013 \text { (5 yr.) } \end{gathered}$ | $\begin{gathered} 1 / 1 / 2009 \text { to } \\ 12 / 31 / 2013 \text { ( } 5 \text { yr.) } \end{gathered}$ |
| Crash Types: |  |  |  |
| Rear Ends - Total | 42 | 54 | 49 |
| Injury (injuries) | 16 (25) | 6 (10) | 19 (29) |
| PDO | 26 | 48 | 30 |
| \% Reduction in Total |  | -10\% |  |
| Approach Turns- Total | 21 | 15 | 25 |
| Injury (injuries) | 5 (5) | 4 (8) | 6 (6) |
| PDO | 16 | 11 | 19 |
| \% Reduction in Total |  | 40\% |  |
| Broadsides- Total | 14 | 12 | 17 |
| Injury (injuries) | 4 (7) | 5 (5) | 5 (8) |
| PDO | 10 | 7 | 12 |
| \% Reduction in Total |  | 29\% |  |
| Pedestrian - Total | 1 | 1 | 3 |
| Fatal (fatalities) | 1 (1) | 0 | 1 (1) |
| Injury (injuries) | 0 | 1 (1) | 0 |
| PDO | 0 | 0 | 2 |
| \% Reduction in Total |  | 66\% |  |

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## Exhibit 3 - Benefit Cost Analysis - Rear End, Approach Turn, Broadside, Pedestrian Crashes Only




## ADT: 30,189 Length: 0.03



## ADT: 30,189 Length: 0.03



## ADT: 32,092 Length: 0.04



## ADT: 32,092 Length: 0.04

## Project Information

Project Name: New Guardrail Installation between Marble and Redstone
Project Description: Install guardrail along SH 133 at select locations

CDOT Region: 3
Location: SH 133
Schedule:

Project Def: 15900
Mile Points: 46.0 to 51.5
Work Start Date: 7/30/2007

County: Gunnison/Pitkin
Length: 5.50 miles
Completion Date: 1/28/2008

Problem Description: As described in the Highway Safety Improvement Program (HSIP) application for this project, the five-year crash history showed a high percentage of severe run-of-the-road type crashes where the vehicle either overturned or struck a formidable object. Combining this with the fact that there have been no injury crashes involving guardrail, suggests that more guardrail along this stretch of highway might significantly reduce the probability of severe crashes. This segment of road has many curves as it follows the Crystal River northward. Crash statistics showed the following number and severity of total crashes (and off road) in the 4 year period between 1/1/2001 and 12/31/2003: PDO - 22 (10), INJ - 7 (5), and FAT 1 (1).

Improvement Description: In 2007 and early 2008, guardrail was installed on the outside of select curves for drop-off protection. The cost of construction was \$703,852.

The HSIP application anticipated that off-road crash types would be impacted by this improvement. The following reductions in crashes were anticipated: fatal crashes - 60\%, injury crashes $-40 \%$, and property damage only $-0 \%$. The initial benefit/cost ratio was estimated to be 4.89 .

## Summary and Findings

The analysis of safety before and after the guardrail was installed along SH 133 showed significant improvement in safety. For this segment of highway, there were 27 total crashes (non-intersection / mainline) during the five-year period before the rail was installed (2002 2006). In the five years after construction (2009-2013), the number of crashes was decreased to 13. In addition, the number of injury and fatal crashes also diminished. During the before period, there was three fatal crashes (overturning, embankment, and a large rock) and a number of injury crashes. The after period experienced one fatal crash (overturning) and a reduction in the number of injury crashes. All of the fatal crashes involved vehicles leaving the road on the right side.

An unusual circumstance has occurred during the after period in that none of the crashes involved hitting the guardrail as either the primary or secondary (or tertiary) occurrence. Since this project is the only significant change in this section of SH 133, the additional guardrail must improve safety in more ways than just creating a physical barrier to more significant hazards. New guardrail (and any related signing) likely increases curve visibility and psychological and cognitive awareness for drivers who may be more cautious in maneuvering through the curves.

The ratio of benefits and cost for this project shows that benefits outweigh costs by a ratio of 21.54 to one, showing that the improvement was certainly justified.

## Results of Safety Analyses

Using VZS, the review of before and after crash records shows a large decrease in the number of crashes; the total number of crashes decreased from 27 during the five-year period (2002 to 2006) before the guardrail was installed (see Table 1 and Exhibit 1) to 13 during the five-year after period (2009 to 2013) (see Table 1 and Exhibit 2). The number of serious crashes also showed a significant decrease:

- Before (2002-2006) - 3 fatal crashes with 4 fatalities (overturning, embankment, and large boulder) and 7 injury crashes with 8 injuries
- After (2009-2013) - 1 fatal crashes with 1 fatalities (overturning - alcohol involved) and 1 injury crash with 1 injury

This decrease in severe crashes occurred along with an increase in traffic volumes on SH 133: 1,520 vehicles per day (vpd) during the before period and $1,810 \mathrm{vpd}$ in the after period. This combination of increased traffic and decreased number of crashes resulted in a decrease in the accident rates:

- Before (2002 - 2006): 1.75 crashes per million vehicle miles of travel (cpmvmt)
- After (2009 - 2013): 0.71 (cpmvmt)

Table 1 - SH 133 (MP 46.0 to MP 51.5) - Results of Overall Crash Analyses

|  | Before | After |
| :---: | :---: | :---: |
| Time Period: | 1/1/2002 to 12/31/2006 (5 yr.) | 1/1/2009 to 12/31/2013 (5 yr.) |
| AADT | 1,520 vpd | 1,810 vpd |
| Filters: | Mainline Only/Non-Intersection | Mainline Only/Non-Intersection |
| Total Crashes | 27 | 13 |
| Fatal Crashes (Fatalities) | 3 (4) | 1 (1) |
| Injury Crashes (Injuries) | 7 (8) | 1 (1) |
| Property Damage Only | 17 | 11 |
| Crash Types: \# (\%) [significance] |  |  |
| Fixed Objects | 17 (63.0\%) [99.81\%] | 9 (69.2\%) [100.00\%] |
| Wild Animal | 5 (18.5\%) | 2 (15.4\%) |
| Overturning | 3 (11.1\%) | 2 (15.4\%) |
| Fixed Object Crashes: \# (\%) [significance] |  |  |
| Large Boulders/Rocks | 5 (29.4\%) | 7 (77.8\%) |
| Embankment | 4 (23.5\%) | 1 (11.1\%) |
| Tree | 4 (23.5\%) | 1 (11.1\%) |
| Guardrail | 2 (11.8\%) | 0 |

The magnitude of safety problems on select highway sections and intersections can be assessed through the use of Safety Performance Function (SPF) methodology. A SPF reflects the complex relationship between exposure (measured in ADT) and the crash count for a section of roadway measured in crashes per mile per year (CPMPY) or for an intersection,
measured in crashes per year. The SPF models provide an estimate for the expected crash frequency and severity for a range of ADT among similar facilities. This allows for an assessment of the magnitude of the safety problem from a frequency standpoint.

Development of the SPF lends itself well to the conceptual formulation of the Levels of Service of Safety (LOSS). The concept of level of service uses qualitative measures that characterize safety of a roadway segment in reference to its expected performance and severity. If the level of safety predicted by the SPF represents a normal or expected number of crashes at a specific level of ADT, then the degree of deviation from the normal can be stratified to represent specific levels of safety.

LOSS-I - Indicates low potential for crash reduction
LOSS-II - Indicates low to moderate potential for crash reduction
LOSS-III - Indicates moderate to high potential for crash reduction
LOSS-IV - Indicates high potential for crash reduction
LOSS boundaries are calibrated by computing the $20^{\text {th }}$ and the $80^{\text {th }}$ percentiles using the Gamma Distribution Probability Density Function. Gradual change in the degree of deviation of the LOSS boundary line from the fitted model mean reflects the observed increase of variability in crashes as ADT increases. LOSS reflects how a segment of roadway or intersection is performing in regard to its expected crash frequency at a specific level of ADT.

SPF plots for both total crashes (see Figure 1) and for fatal and injury crashes (see Figure 2) also reflect this improvement in the crash record. LOSS for both Total and Injury/Fatal crashes improved from LOSS III during the before period to the LOSS I range in the after period (see Table 2). The No Build After crashes were estimated using the increase in the median of the SPF for total crashes (1.12) and for injury/fatal crashes (1.03) found in Table 2.

A more detailed review of the before and after crash record (see Table 3) reveals that this pattern of significant improvement in safety also occurred for the types of crashes that are most directly affected by the guardrail: off-road right and off-road left. Table 3 shows that poor roadway conditions (snowy/icy/slushy) are a contributing factor in more than half of these crashes in both the before and after periods. For the No Build After scenario, the same SPF increase factors were used - PDO (Total SPF change = 1.12) and Injury/Fatal (SPF change = 1.03). The unusual circumstance during the after period is that none of the crashes involved hitting the guardrail as either the primary or secondary (or tertiary) occurrence. Since this project is the only significant change in this section of SH 133, the additional guardrail must improve safety in more ways than just creating a physical barrier to more significant hazards. New guardrail (and any related signing and reflectors) likely increases curve visibility and delineation as well as psychological and cognitive awareness for drivers who may then be more cautious in maneuvering through the curves.

Figure 1 -SPF for Total Crashes
SH 133 (MP 46.0 to MP 51.5)
Before: $\mathbf{2 0 0 2}$ to 2006 After: 2009 to 2013


Note: Safety Performance Function (SPF) Model: Colorado - Rural Mountainous 2-Lane Undivided Highway (2002)

Figure 2-SPF for Injury and Fatal Crashes
SH 133 (MP 46.0 to MP 51.5)
Before: $\mathbf{2 0 0 2}$ to 2006 After: 2009 to 2013


Note: Safety Performance Function (SPF) Model: Colorado - Rural Mountainous 2-Lane Undivided Highway (2002)

Table 2 - SH 133 (MP 46.0 to MP 51.5) - Safety Performance Function (SPF)

|  | Before | After | No Build After |  |
| :--- | :---: | :---: | :---: | :---: |
| EB Correction: | Yes | No | Yes |  |
| SPF Graph | Rural, Mountainous, <br> 2-Lane, Undivided <br> Highway | Rural, Mountainous, <br> 2-Lane, Undivided <br> Highway | Rural, Mountainous, <br> 2-Lane, Undivided <br> Highway |  |
| AADT | 1,520 vpd | 1,810 vpd | 1,810 vpd |  |
| Total Crashes: | LOSS III | LOSS I | LOSS III |  |
| LOSS | 0.91 | 0.47 | 1.08 |  |
| CPMPY | 0.81 | 0.96 | 0.96 |  |
| Mean CPMPY | 1.12 | 0.49 | 1.12 |  |
| Proportion of Mean |  |  |  |  |
| Fatal \& Injury Crashes: | LOSS III | LOSS I | LOSS III |  |
| LOSS | 0.34 | 0.07 | 0.39 |  |
| CPMPY | 0.33 | 0.38 | 0.38 |  |
| Mean CPMPY | 1.03 | 0.18 | 1.03 |  |
| Proportion of Mean |  |  |  |  |

Table 3 - SH 133 (MP 46.0 to MP 51.5) - Results of Off-Road Right and Left Crash Analyses

|  | Before | After | No Build After |
| :---: | :---: | :---: | :---: |
| Time Period: | $1 / 1 / 2002$ to <br> $12 / 31 / 2006(5 \mathrm{yr})$. | $1 / 1 / 2009$ to <br> $12 / 31 / 2013(5 \mathrm{yr})$. | $1 / 1 / 2009$ to <br> $12 / 31 / 2013(5 \mathrm{yr})$. |
| AADT | $1,520 \mathrm{vpd}$ | $1,810 \mathrm{vpd}$ | $1,810 \mathrm{vpd}$ |
| Crash Types: |  |  |  |
| Off-Road Right \& Left - | $\mathbf{1 9}$ | $\mathbf{7}$ | $\mathbf{2 1}$ |
| Fatal (fatalities) | $3(4)$ | $1(1)$ | $3(4)$ |
| Injury (injuries) | $6(6)$ | 0 | $6(6)$ |
| PDO | 10 | 6 | 12 |
| \% Reduction in Total - |  | $75 \% / 100 \% / 50 \%$ |  |
| FAT / INJ / PDO |  | 7 | $\mathrm{n} / \mathrm{a}$ |
| Road Conditions | 19 | 2 | $\mathrm{n} / \mathrm{a}$ |
| Dry | 6 | 0 | $\mathrm{n} / \mathrm{a}$ |
| Wet | 2 | 5 | $\mathrm{n} / \mathrm{a}$ |
| Snowy/Icy/Slushy | 11 |  |  |

Vision Zero Suite (VZS) includes benefit/cost (B/C) analyses within its procedures. The results of the B/C analysis are shown in Exhibit 3 for off-road right and left crashes. The B/C ratio for the project is 21.54 , showing that the improvement was certainly justified.

Exhibit 3 - Benefit Cost Analysis - Off-Road Right Crashes Only

|  |  | Colorado Department of Transportation DiExSys ${ }^{\text {TM }}$ Roadway Safety Systems Economic Analysis Report |  |  |  |  | Job | 2015 | 06/25/2015 025092132 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Locatio | 133A |  |  |  | :46.00 | End:51.50 | From:01/01/2002 | To:12/3 | 1/2006 |
| Benefit Cost Ratio Calculations |  |  |  |  |  |  |  |  |  |
| Accidents |  |  | Projected Accidents and Reduction Factors |  |  |  | Other Information |  |  |
| $\begin{gathered} \text { PDO: } \\ \text { INJ: } \\ \text { FAT: } \end{gathered}$ | 1263 | 6 :Injured 4 :Killed | Weighted PDO: <br> Weighted INJ: <br> Weighted FAT: | 2.95 | 50\%:A | $F$ for PDO | Cost of PDO: | \$ | 9,300 |
|  |  |  |  | 1.47 | 100\%:A | F for INJ | Cost of INJ: | \$ | 80,700 |
|  |  |  |  | 0.98 | 75\%:A | F for FAT | Cost of FAT: | \$ 1, | ,500,000 |
| B/C Weighted Year Factor: |  |  |  | 5.00 | 67\%:W | ighted ARF | Interest Rate: | 5\% |  |
|  |  |  |  |  |  |  | T Growth Factor: | 2.0\% |  |
| Cost:\$ 703,852 |  |  |  |  |  |  | Service Life: | 20 |  |
| From: 01/01/2009 |  |  |  |  |  |  | Recovery Factor: | 0.080 |  |
| To: 12/31/2013 |  |  | Days: 1826 |  |  | Annu | aintenance Cost: | \$ | 1,000 |
| Benefit Cost Ratio: 21.54 |  |  | (B/C Based on Injury Numbers : PDO/Injured/Killed) |  |  |  |  |  |  |
| Type of Improvement: Guardrail Installation - Off-Road Right \& Left Crashes Only Special Notes: Latest NSC Crash Costs |  |  |  |  |  |  |  |  |  |



## ADT: 1,521 Length: 5.54



## ADT: 1,521 Length: 5.54



## ADT: 1,810 Length: 5.54



## ADT: 1,810 Length: 5.54

## Project Information

Project Name: US 50 / Purcell Boulevard
Project Description: Turn lanes, upgrade signal, signal timing
CDOT Region: $2 \quad$ Project Def: 16005
Location: US $50 \quad$ Mile Points: 309.78

County: Pueblo
Length: N/A
Completion Date: 11/13/2007

Problem Description: As described in the Highway Safety Improvement Program (HSIP) application for this project, the five-year crash history showed a higher than expected number of rear-end type crashes. This is due to a capacity problem which is caused because the intersection has to run split phase timing due to geometry problems.

Improvement Description: In 2007, the intersection was widened to construct dual left-turn lanes, two through lanes, and a right turn lane on Purcell. This corrected the geometry problem so the intersection no longer needed to run split phase timing. The cost of construction was \$1,302,499.

The HSIP application anticipated that four crash types would be impacted by this improvement: rear-end, approach turn, broadside, and pedestrian type crashes. It was anticipated that there would be a $35 \%$ crash reduction for these crash types. The initial benefit/cost ratio was estimated to be 1.77.

## Summary and Findings

The analysis of safety before and after the geometry and signal was upgraded at US 50 and Purcell Boulevard showed safety improvements. For this intersection, there were 109 total crashes during the five-year period before the upgrades (2002-2006). In the five years after construction (2008-2012), the number of crashes was decreased to 87 . Since daily volumes continued to increase throughout the study period, the crash rate was reduced. In addition, the number of injuries also diminished although there was an additional fatality in the after period.

The signal and geometry upgrade was responsible for decreases in the number and severity of rear end, approach turn, and broadside. The ratio of benefits and cost for this project shows that benefits were less than costs by a ratio of 0.74 to one. However, this $B / C$ analysis includes a tragic but random event that was totally unrelated to the signal or modified intersection geometry. Removing the fatality gives a B/C ratio is 4.00 , showing that the improvement was likely justified.

## Results of Safety Analyses

Using VZS, the review of before and after crash records shows a decrease in the number of crashes; the total number of crashes decreased from 109 during the five-year period (2002 to 2006) before the signal was upgraded (see Table 1 and Exhibit 1) to 87 during the five-year after period (2008 to 2012) (see Table 1 and Exhibit 2). The number of serious crashes showed only a slight decrease, although the number of injuries decreased significantly:

- Before (2002 - 2006) - no fatal crashes and 35 injury crashes with 58 injuries
- After (2008-2012) - 1 fatal crash with 1 fatality (rear-end) and 30 injury crashes with 43 injuries

The fatal crash in the after period occurred in unusual circumstances. An eastbound vehicle was stopped at a green light for a southbound fire truck traveling with lights and sirens. A second eastbound vehicle did not stop and hit the stopped vehicle at a high speed in a rear-end type crash. This decrease in injuries occurred in spite of a modest increase in traffic volumes at the intersection.

Table 1 - Results of Overall Crash Analyses

| SH50A, MP 309.76-309.80 | Before | After |
| :--- | :---: | :---: |
| Time Period: | $1 / 1 / 2002$ to $12 / 31 / 2006$ (5 yr.) | $1 / 1 / 2008$ to 12/31/2012 (5 yr.) |
| AADT (SH 50/Purcell Blvd) | $29,131 / 16,500 \mathrm{vpd}$ | $30,488 / 16,500 \mathrm{vpd}$ |
| Filters: | At Intersection <br> Intersection Related | At Intersection <br> Intersection Related |
| Total Crashes | 109 | 87 |
| Fatal Crashes (Fatalities) | 0 | $1(1)$ |
| Injury Crashes (Injuries) | $32(58)$ | $30(43)$ |
| Property Damage Only | 77 | 56 |
| Crash Types: \# (\%) [significance] | $84(77.1 \%)[100.00 \%]$ | $63(72.4 \%)[100.00 \%]$ |
| Rear End | $11(10.1 \%)$ | $6(6.9 \%)$ |
| Broadside | $8(7.3 \%)$ | $4(4.6 \%)$ |
| Sideswipe Same | $1(0.9 \%)$ | $9(10.3 \%)$ |
| Approach Turn |  |  |

The magnitude of safety problems on select highway sections and intersections can be assessed through the use of Safety Performance Function (SPF) methodology. A SPF reflects the complex relationship between exposure (measured in ADT) and the crash count for a section of roadway measured in crashes per mile per year (CPMPY) or for an intersection, measured in crashes per year. The SPF models provide an estimate for the expected crash frequency and severity for a range of ADT among similar facilities. This allows for an assessment of the magnitude of the safety problem from a frequency standpoint.

Development of the SPF lends itself well to the conceptual formulation of the Levels of Service of Safety (LOSS). The concept of level of service uses qualitative measures that characterize safety of a roadway segment in reference to its expected performance and severity. If the level of safety predicted by the SPF represents a normal or expected number of crashes at a specific

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level of ADT, then the degree of deviation from the normal can be stratified to represent specific levels of safety.

LOSS-I - Indicates low potential for crash reduction
LOSS-II - Indicates low to moderate potential for crash reduction
LOSS-III - Indicates moderate to high potential for crash reduction
LOSS-IV - Indicates high potential for crash reduction
LOSS boundaries are calibrated by computing the $20^{\text {th }}$ and the $80^{\text {th }}$ percentiles using the Gamma Distribution Probability Density Function. Gradual change in the degree of deviation of the LOSS boundary line from the fitted model mean reflects the observed increase of variability in crashes as ADT increases. LOSS reflects how a segment of roadway or intersection is performing in regard to its expected crash frequency at a specific level of ADT.

SPF plots for both total crashes (see Figure 1) and for fatal and injury crashes (see Figure 2) also reflect this improvement in the crash record. LOSS improved to the LOSS II range from LOSS III for total crashes and remained within the LOSS III range for Injury/Fatal crashes in the after period (see Table 2).

Figure 1 - SPF for Total Crashes
US 50 / Purcell Blvd
Before: 2002 to 2006 After: 2008 to 2012


Note: Safety Performance Function (SPF) Model - Urban 4-Lane Divided Signalized 4-Leg Intersection

Figure 2 - SPF for Injury and Fatal Crashes US 50 I Purcell Blvd
Before: 2002 to 2006 After: 2008 to 2012


Note: Safety Performance Function (SPF) Model - Urban 4-Lane Divided Signalized 4-Leg Intersection

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Table 2 - Safety Performance Function (SPF)

| SH50A, MP 309.76-309.80 | Before | After | No Build After |  |
| :--- | :---: | :---: | :---: | :---: |
| EB Correction: | Yes | No | Yes |  |
| SPF Graph | Urban, 4-lane, <br> Divided, Signalized, <br> 4-Leg Intersection | Urban, 4-lane, <br> Divided, Signalized, <br> 4-Leg Intersection | Urban, 4-lane, <br> Divided, Signalized, <br> 4-Leg Intersection |  |
| Total Crashes: | LOSS III | LOSS II | LOSS III |  |
| LOSS | 21.51 | 17.40 | 22.26 |  |
| CPY | 18.05 | 18.71 | 18.71 |  |
| Mean CPY | 1.19 | 0.93 | 1.19 |  |
| Proportion of Mean |  |  |  |  |
| Fatal \& Injury Crashes: | LOSS III | LOSS III | LOSS III |  |
| LOSS | 6.18 | 6.20 | 6.40 |  |
| CPY | 5.29 | 5.47 | 5.47 |  |
| Mean CPY | 1.17 | 1.13 | 1.17 |  |
| Proportion of Mean |  |  |  |  |

A more detailed review of the before and after crash record reveals that a significant improvement in safety can be attributed to the upgrade of the signal, with the exception of the previously mentioned fatality. Table 3 shows a comparison of three types of crashes that are most directly affected by the improvement: rear end, approach turn, and broadside. The No Build After crashes were estimated using the increase in the median of the SPF for total crashes found in Table 2 (increase is $1.04=18.71 / 18.02$ ).

Table 3 - Results of Crash Analyses

| SH50A, MP 309.76-309.80 | Before | After | No Build After |
| :---: | :---: | :---: | :---: |
| Time Period: | $\begin{gathered} \hline \text { 1/1/2002 to } \\ 12 / 31 / 2006 \text { (5 yr.) } \end{gathered}$ | $\begin{gathered} \hline 1 / 1 / 2008 \text { to } \\ 12 / 31 / 2012 \text { (5 yr.) } \\ \hline \end{gathered}$ | $\begin{gathered} \hline 1 / 1 / 2008 \text { to } \\ 12 / 31 / 2012 \text { (5 yr.) } \\ \hline \end{gathered}$ |
| Crash Types: |  |  |  |
| Rear Ends - Total | 84 | 63 | 87 |
| Fatal (fatalities) | 0 | 1 (1) | 0 |
| Injury (injuries) | 28 (47) | 22 (31) | 29 (49) |
| PDO | 56 | 40 | 58 |
| \% Reduction in Total |  | 18\% |  |
| Approach Turns- Total | 1 | 9 | 1 |
| Injury (injuries) | 1 (7) | 1 (2) | 1 (7) |
| PDO | 0 | 8 | 0 |
| \% Reduction in Total |  | -800\% |  |
| Broadsides- Total | 11 | 6 | 11 |
| Injury (injuries) | 3 (4) | 5 (8) | 3 (4) |
| PDO | 8 | 1 | 8 |
| \% Reduction in Total |  | 45\% |  |

Vision Zero Suite (VZS) includes benefit/cost (B/C) analyses within its procedures. The results of the B/C analysis are shown in Exhibit 3 for the impacted crash types. The after period had a rear-end fatality when there was no fatality in the no action after. To account for the cost of the fatality in the B/C analysis, the cost of a fatality was added in to the overall cost of the project. As shown, the B/C ratio for rear end, approach turn, and broadside crashes is 0.74 , showing that the improvement may not have been justified. However, it may be worth discounting the fatality due to its unique nature. Since the crash occurred on a green light with traffic stopped for a fire truck, it likely unrelated to the signal or modified intersection geometry. Exhibit 4 provides the results of the B/C analysis without considering the fatality. As shown, the B/C ratio is 4.00 , showing that the improvement may have been justified.

## Exhibit 3 - Benefit Cost Analysis - Rear End, Approach Turn, Broadside Crashes Only



## Exhibit 4 - Benefit Cost Analysis - Rear End, Approach Turn, Broadside Crashes Only, Excluding Fatality




## ADT: 29,131 Length: 0.03



## ADT: 29,131 Length: 0.03



## ADT: 30,448 Length: 0.04



## ADT: 30,448 Length: 0.04

## Project Information

Project Name: SH 45 / Red Creek Springs Rd
Project Description: Add right turn lanes, replace existing signal
CDOT Region: 2 Project Def: 16006 County: Pueblo
Location: SH 45 Mile Points: 3.95 Length: N/A
Schedule: $\quad$ Work Start Date: 1/12/2009 Completion Date: 11/10/2009
Problem Description: As described in the Highway Safety Improvement Program (HSIP) application for this project, the three-year crash history showed a higher than expected number of rear-end type crashes. This is due to a capacity problem which is caused by the side streets being too narrow to accommodate turn lanes. As a result, side street green time has to be extended, which causes progression problems on SH 45. In addition, the corner radii are small.

Improvement Description: In 2009, the Red Creek Springs Road was widened to add an eastbound right-turn lane. SH 45 was widened for a southbound right-turn lane. The cost of construction was $\$ 779,500$.

The HSIP application anticipated that there would be a 15\% crash reduction for all intersection crashes. The initial benefit/cost ratio was estimated to be 1.18.

## Summary and Findings

The analysis of safety before and after the geometry and signal was upgraded at US 45 and Red Creek Springs Road showed no safety improvements. For this intersection, there were 47 total crashes during the four-year period before the upgrades (2005-2008). In the four years after construction (20010-2013), the number of crashes decreased to 46 . Since daily volumes continued to increase throughout the study period, the crash rate was reduced. However, the number of injuries increase from 20 in the before period to 34 in the after period.

The signal and geometry upgrade was responsible for decreases in the number and severity of broadside, and pedestrian crashes. However, there was a significant increase in the severity of rear-end crashes. The overall ratio of benefits and cost when including approach turns, broadsides, rear-ends, and pedestrian crashes show the resulting B/C ratio of the project was 0.08 . The result is an improvement whose safety benefits did not outweigh the cost.

## Results of Safety Analyses

Using VZS, the review of before and after crash records shows a decrease in the number of crashes; the total number of crashes decreased from 47 during the four-year period ( 2005 to 2008) before the signal was upgraded and the geometry was improved (see Table 1 and Exhibit 1) to 46 during the four-year after period (2010 to 2013) (see Table 1 and Exhibit 2). The number of serious crashes increased while the PDO crashes decreased.

Table 1 - Results of Overall Crash Analyses

| SH45, MP 3.93-3.97 | Before | After |
| :--- | :---: | :---: |
| Time Period: | $1 / 1 / 2005$ to $12 / 31 / 2008$ (4 yr.) | $1 / 1 / 2010$ to 12/31/2013 (4 yr.) |
| AADT (SH 45/RCS Rd) | $25,500 / 5,500 \mathrm{vpd}$ | $27,000 / 5,500 \mathrm{vpd}$ |
| Filters: | At Intersection <br> Intersection Related | At Intersection <br> Intersection Related |
| Total Crashes | 47 | 46 |
| Fatal Crashes (Fatalities) | 0 | 0 |
| Injury Crashes (Injuries) | $13(20)$ | $20(34)$ |
| Property Damage Only | 34 | 26 |
| Crash Types: \# (\%) [significance] |  | $21(45.7 \%)$ |
| Rear End | $17(36.2 \%)$ | $14(30.4 \%)[97.64 \%]$ |
| Approach Turn | $12(25.5 \%)$ | $4(8.7 \%)$ |
| Broadside | $11(23.4 \%)[95.04 \%]$ | 0 |
| Pedestrian | $3(6.4 \%)[99.06 \%]$ |  |

The magnitude of safety problems on select highway sections and intersections can be assessed through the use of Safety Performance Function (SPF) methodology. A SPF reflects the complex relationship between exposure (measured in ADT) and the crash count for a section of roadway measured in crashes per mile per year (CPMPY) or for an intersection, measured in crashes per year. The SPF models provide an estimate for the expected crash frequency and severity for a range of ADT among similar facilities. This allows for an assessment of the magnitude of the safety problem from a frequency standpoint.

Development of the SPF lends itself well to the conceptual formulation of the Levels of Service of Safety (LOSS). The concept of level of service uses qualitative measures that characterize safety of a roadway segment in reference to its expected performance and severity. If the level of safety predicted by the SPF represents a normal or expected number of crashes at a specific level of ADT, then the degree of deviation from the normal can be stratified to represent specific levels of safety.

LOSS-I - Indicates low potential for crash reduction
LOSS-II - Indicates low to moderate potential for crash reduction
LOSS-III - Indicates moderate to high potential for crash reduction
LOSS-IV - Indicates high potential for crash reduction
LOSS boundaries are calibrated by computing the $20^{\text {th }}$ and the $80^{\text {th }}$ percentiles using the Gamma Distribution Probability Density Function. Gradual change in the degree of deviation of the LOSS boundary line from the fitted model mean reflects the observed increase of variability
in crashes as ADT increases. LOSS reflects how a segment of roadway or intersection is performing in regard to its expected crash frequency at a specific level of ADT.

SPF plots for both total crashes (see Figure 1) and for fatal and injury crashes (see Figure 2) also reflect the improvement in the total crash record while showing the increase in severe crashes. LOSS improved within the LOSS III range for total crashes and increased to LOSS IV for Injury/Fatal crashes in the after period (see Table 2).

Figure 1 - SPF for Total Crashes
US 45 I Red Creek Springs Road
Before: 2005 to 2008 After: 2010 to 2013


Note: Safety Performance Function (SPF) Model - Urban 4-Lane Divided Signalized 4-Leg Intersection

Figure 2 - SPF for Injury and Fatal Crashes US 45 I Red Creek Springs Road
Before: 2005 to 2008 After: 2010 to 2013


Note: Safety Performance Function (SPF) Model - Urban 4-Lane Divided Signalized 4-Leg Intersection

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Table 2 - Safety Performance Function (SPF)

| SH45, MP 3.93-3.97 | Before | After | No Build After |  |
| :--- | :---: | :---: | :---: | :---: |
| EB Correction: | Yes | No | Yes |  |
| SPF Graph | Urban, 4-lane, <br> Divided, Signalized, <br> 4-Leg Intersection | Urban, 4-lane, <br> Divided, Signalized, <br> 4-Leg Intersection | Urban, 4-lane, <br> Divided, Signalized, <br> 4-Leg Intersection |  |
| Total Crashes: | LOSS III | LOSS III | LOSS III |  |
| LOSS | 11.34 | 11.50 | 11.93 |  |
| CPY | 9.29 | 9.78 | 9.78 |  |
| Mean CPY | 1.22 | 1.18 | 1.22 |  |
| Proportion of Mean |  |  |  |  |
| Fatal \& Injury Crashes: | LOSS III | LOSS IV | LOSS IIII |  |
| LOSS | 3.11 | 5.00 | 3.26 |  |
| CPY | 2.87 | 3.02 | 3.02 |  |
| Mean CPY | 1.08 | 1.66 | 1.08 |  |
| Proportion of Mean |  |  |  |  |

A more detailed review of the before and after crash record reveals that some improvement in the number of crashes can be attributed to the upgrade of the signal and modified geometry, although the severity of crashes increased. Table 3 shows a comparison of four types of crashes that are most directly affected by the improvement: rear end, approach turn, broadside, and pedestrian. The No Build After crashes were estimated using the increase in the mean predicted by the SPF reflecting AADT for the after period, the SPF for total crashes found in Table 2 (increase is $1.05=9.78 / 9.29$ ). As shown, the pedestrian and broadside crashes had a significant decrease in number of crashes. However, the number of rear-end crashes resulting in injuries increased.

Table 3 - Results of Crash Analyses

| SH45, MP 3.93-3.97 | Before | After | No Build After |
| :---: | :---: | :---: | :---: |
| Time Period: | $\begin{gathered} \hline 1 / 1 / 2005 \text { to } \\ 12 / 31 / 2008 \text { (4 yr.) } \\ \hline \end{gathered}$ | $\begin{gathered} \hline 1 / 1 / 2010 \text { to } \\ 12 / 31 / 2013 \text { (4 yr.) } \\ \hline \end{gathered}$ | $\begin{gathered} \hline 1 / 1 / 2010 \text { to } \\ 12 / 31 / 2013 \text { (4 yr.) } \\ \hline \end{gathered}$ |
| Crash Types: |  |  |  |
| Rear Ends - Total | 17 | 21 | 18 |
| Injury (injuries) | 0 | 7 (12) | 0 |
| PDO | 17 | 14 | 18 |
| \% Reduction in Total |  | -16\% |  |
| Approach Turns- Total | 12 | 14 | 13 |
| Injury (injuries) | 4 (7) | 6 (12) | 4 (7) |
| PDO | 8 | 8 | 9 |
| \% Reduction in Total |  | -8\% |  |
| Broadsides- Total | 11 | 4 | 11 |
| Injury (injuries) | 4 (8) | 4 (6) | 4 (8) |
| PDO | 7 | 0 | 7 |
| \% Reduction in Total |  | 64\% |  |
| Pedestrians- Total | 3 | 0 | 3 |
| Injury (injuries) | 3 (3) | 0 | 3 (3) |
| PDO | 0 | 0 | 0 |
| \% Reduction in Total |  | 100\% |  |

Vision Zero Suite (VZS) includes benefit/cost (B/C) analyses within its procedures. The results of the B/C analysis are shown in Exhibit 3 for the impacted crash types. The after period had a significantly larger number of Injury crashes than predicted. To account for the increase in Injury crashes in the B/C analysis, the cost of the injuries was added in to the overall cost of the project. As shown, the B/C ratio for rear end, approach turn, broadside, and pedestrian crashes is 0.08 , showing that the safety benefits of the improvement did not outweigh the cost.

## Exhibit 3 - Benefit Cost Analysis - Rear End, Approach Turn, Broadside, Pedestrian Crashes Only




## ADT: 33,650 Length: 0.04



## ADT: 33,650 Length: 0.04



## ADT: 27,000 Length: 0.04



## ADT: 27,000 Length: 0.04

## Project Information

Project Name: Signal at Industrial / Purcell
Project Description: Install a new signal
CDOT Region: 2 Project Def: 16010 County: Pueblo
Location: Industrial Boulevard/Purcell Boulevard
Schedule: $\quad$ Work Start Date: approx. 5/2008 Completion Date: approx. 6/2009
Problem Description: As described in the Highway Safety Improvement Program (HSIP) application for this project, the five-year crash history showed a higher than expected number of broadside type crashes. The intersection was unsignalized but met signal warrants.

Improvement Description: In late 2008 and early 2009, a traffic signal was installed at this intersection. The cost of construction was $\$ 391,768$.

The HSIP application anticipated that four crash types would be impacted by this improvement: rear-end, approach turn, broadside, and sideswipe same direction type crashes. It was anticipated that there would be a $25 \%$ crash reduction for these crash types. The initial benefit/cost ratio was estimated to be 1.12.

## Summary and Findings

The analysis of safety before and after the signal was installed at Industrial Boulevard and Purcell Boulevard showed no safety improvements. For this intersection, there were 14 total crashes during the three-year period before the upgrades (2005-2007). In the three years after construction (2011-2013), the number of crashes increased to 22. Despite daily volumes increasing slightly throughout the study period, the crash rate increased. In addition, the number of injury and fatal crashes also increased.

The signal installation was responsible for a moderate decrease in the number broadside crashes, but the after period showed an increase in the number and severity of approach turn, rear-end, and sideswipe same direction crashes. The result is an improvement that was likely not justified from the standpoint of safety. This outcome suggests that considering relatively low traffic volumes at this location a modern roundabout would be a preferred countermeasure.

## Results of Safety Analyses

Using VZS, the review of before and after crash records shows an increase in the number of crashes; the total number of crashes increased from 14 during the three-year period (2005 to 2007) before the signal was installed (see Table 1 and Exhibit 1) to 22 during the three-year after period (2011 to 2013) (see Table 1 and Exhibit 2).

This increase in crashes occurred along with a modest increase in traffic volumes at the intersection. This combination of increased traffic and increased number of crashes also resulted in a increase in the accident rates:

- Before (2005-2007): 1.15 crashes per million entering vehicles (cpmev)
- After (2011 - 2013): 1.75 (cpmev)


## Table 1 - Results of Overall Crash Analyses

| Purcell/Industrial | Before | After |
| :--- | :---: | :---: |
| Time Period: | $1 / 1 / 2005$ to $12 / 31 / 2007$ (3 yr.) | $1 / 1 / 2011$ to $12 / 31 / 2013$ (3 yr.) |
| AADT (Purcell/Industrial) | $6,000 / 5,100$ vpd | $6,000 / 5,500$ vpd |
| Filters: | At Intersection <br> Intersection Related | At Intersection <br> Intersection Related |
| Total Crashes | $\mathbf{1 4}$ | $\mathbf{2 2}$ |
| Fatal Crashes (Fatalities) | 0 | $1(1)$ |
| Injury Crashes (Injuries) | $2(2)$ | $5(9)$ |
| Property Damage Only | 12 | 16 |
| Crash Types: \# (\%) [significance] | $6(42.9 \%)[98.27 \%]$ | $4(18.2 \%)$ |
| Broadside | $3(21.4 \%)$ | $4(18.2 \%)$ |
| Sideswipe Same | $2(14.3 \%)$ | $4(18.2 \%)$ |
| Rear End | $1(7.1 \%)$ | $9(40.9 \%)[99.51 \%]$ |
| Approach Turn |  |  |

The magnitude of safety problems on select highway sections and intersections can be assessed through the use of Safety Performance Function (SPF) methodology. A SPF reflects the complex relationship between exposure (measured in ADT) and the crash count for a section of roadway measured in crashes per mile per year (CPMPY) or for an intersection, measured in crashes per year. The SPF models provide an estimate for the expected crash frequency and severity for a range of ADT among similar facilities. This allows for an assessment of the magnitude of the safety problem from a frequency standpoint.

Development of the SPF lends itself well to the conceptual formulation of the Levels of Service of Safety (LOSS). The concept of level of service uses qualitative measures that characterize safety of a roadway segment in reference to its expected performance and severity. If the level of safety predicted by the SPF represents a normal or expected number of crashes at a specific level of ADT, then the degree of deviation from the normal can be stratified to represent specific levels of safety.

LOSS-I - Indicates low potential for crash reduction
LOSS-II - Indicates low to moderate potential for crash reduction
LOSS-III - Indicates moderate to high potential for crash reduction
LOSS-IV - Indicates high potential for crash reduction
Gradual change in the degree of deviation of the LOSS boundary line from the fitted model mean reflects the observed increase of variability in crashes as ADT increases. LOSS reflects how a segment of roadway or intersection is performing in regard to its expected crash frequency at a specific level of ADT.

SPF plots for both total crashes (see Figures 1 and 3) and for fatal and injury crashes (see Figure 2 and 4) reflect no improvement in the crash record. LOSS worsened within the LOSS IV range for total crashes in the after period, and Injury/Fatal crashes worsened to LOSS IV from LOSS II in the after period (see Table 2). Figure 5 and 6 show the SPF plots for the no action after period.

Figure 1 - SPF for Total Crashes - Before Industrial Blvd/Purcell Blvd 2005 to 2007


Note: Safety Performance Function (SPF) Model - Urban 4-Lane Divided Unsignalized 4-Leg Intersection

Figure 2 - SPF for Injury and Fatal Crashes - Before Industrial Blvd/Purcell Blvd 2005 to 2007


Note: Safety Performance Function (SPF) Model - Urban 4-Lane Divided Unsignalized 4-Leg Intersection

Figure 3 - SPF for Total Crashes - After Industrial Blvd/Purcell Blvd 2011 to 2013


Note: Safety Performance Function (SPF) Model - Urban 4-Lane Divided Signalized 4-Leg Intersection

Figure 3 - SPF for Injury and Fatal Crashes - After Industrial BlvalPurcell Blvd

2011 to 2013


Note: Safety Performance Function (SPF) Model - Urban 4-Lane Divided Signalized 4-Leg Intersection

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Figure 5 - SPF for Total Crashes - No Action After Industrial Blvd/Purcell BIvd

2011 to 2013


Note: Safety Performance Function (SPF) Model - Urban 4-Lane Divided Unsignalized 4-Leg Intersection

Figure 6 - SPF for Injury and Fatal Crashes - No Action After Industrial Blvd/Purcell Blvd 2011 to 2013


Note: Safety Performance Function (SPF) Model - Urban 4-Lane Divided Unsignalized 4-Leg Intersection

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Table 2 - Safety Performance Function (SPF)

| Purcell/Industrial | Before | After | No Build After |  |
| :--- | :---: | :---: | :---: | :---: |
| EB Correction: | Yes | No | Yes |  |
| SPF Graph | Urban, 4-lane, <br> Divided, <br> Unsignalized, 4-Leg <br> Intersection | Urban, 4-lane, <br> Divided, Signalized, <br> 4-Leg Intersection | Urban, 4-lane, <br> Divided, <br> Unsignalized, 4-Leg <br> Intersection |  |
| Total Crashes: | LOSS IV | LOSS IV | LOSS IV |  |
| LOSS | 3.64 | 7.33 | 3.75 |  |
| CPY | 1.90 | 1.56 | 1.96 |  |
| Mean CPY | 1.92 | 4.70 | 1.92 |  |
| Proportion of Mean |  |  |  |  |
| Fatal \& Injury Crashes: | LOSS II | LOSS IV | LOSS II |  |
| LOSS | 0.76 | 2.00 | 0.78 |  |
| CPY | 0.86 | 0.41 | 0.88 |  |
| Mean CPY | 0.88 | 4.88 | 0.88 |  |
| Proportion of Mean |  |  |  |  |

A more detailed review of the before and after crash record reveals that very little improvement in safety can be attributed to the installation of the signal. Table 3 shows a comparison of four types of crashes that are most directly affected by the improvement: rear end, approach turn, broadside, and sideswipe same direction. The No Build After crashes were estimated using the increase in the median of the SPF for total crashes found in Table 2 (increase is $1.03=$ 1.96/1.90). As shown, the number of broadsides crashes decreased (although the severity increased) and all other crash types had an increase in number of crashes.

Table 3 - Results of Crash Analyses

| Purcell/Industrial | Before | After | No Build After |
| :---: | :---: | :---: | :---: |
| Time Period: | $\begin{gathered} \hline 1 / 1 / 2005 \text { to } \\ 12 / 31 / 2007 \text { (3 yr.) } \end{gathered}$ | $\begin{gathered} \text { 1/1/2011 to } \\ 12 / 31 / 2013 \text { (3 yr.) } \end{gathered}$ | $\begin{gathered} \text { 1/1/2011 to } \\ 12 / 31 / 2013 \text { (3 yr.) } \end{gathered}$ |
| Crash Types: |  |  |  |
| Broadsides- Total | 6 | 4 | 6 |
| Injury (injuries) | 0 | 1 (3) | 0 |
| PDO | 6 | 3 | 6 |
| \% Reduction in Total |  | 33\% |  |
| Sideswipe Same Direction - Total | 3 | 4 | 3 |
| Injury (injuries) | 0 | 0 | 0 |
| PDO | 3 | 4 | 3 |
| \% Reduction in Total |  | -33\% |  |
| Rear Ends - Total | 2 | 4 | 2 |
| Injury (injuries) | 0 | 2 (4) | 0 |
| PDO | 2 | 2 | 2 |
| \% Reduction in Total |  | -100\% |  |
| Approach Turns- Total | 1 | 9 | 1 |
| Fatal (fatalities) | 0 | 1 (1) | 0 |
| Injury (injuries) | 0 | 2 (2) | 0 |
| PDO | 1 | 6 | 1 |
| \% Reduction in Total |  | -800\% |  |

There was no improvement in safety amongst the combined impacted crash types in the number of PDO, Injury, or Fatality crashes, showing that the improvement was likely not justified from a safety standpoint.




| Lighting Conditions |  |
| ---: | ---: |
| Daylight: | 14 |
| Dawn or Dusk: | 5 |
| Dark - Lighted: | 3 |
| Dark - Unlighted: | 0 |
| Unknown: | 0 |
| Total: | $\mathbf{2 2}$ |


| Number of Vehicles |  |
| ---: | ---: |
| One Vehicle: | 0 |
| Two Vehicles: | 22 |
| Three or More: | 0 |
| Unknown: | 0 |
| Total: | $\mathbf{2 2}$ |


| Location: Accident History for INDUS |
| :--- |
| Severity   <br> PDO: 16  <br> INJ: 5 9 :Injured <br> FAT: 1 1 :Killed <br> Total: $\mathbf{2 2}$  |


| - Crash Type |  |  |  |
| :---: | :---: | :---: | :---: |
| Overturning: | 0 | Bridge Abutment: | 0 |
| Other Non Collision: | 0 | Column/Pier: | 0 |
| Pedestrians: | 0 | Culvert/Headwall: | 0 |
| Broadside: | 4 | Embankment: | 0 |
| Head On: | 0 | Curb: | 1 |
| Rear End: | 4 | Delineator Post: | 0 |
| Sideswipe (Same): | 4 | Fence: | 0 |
| Sideswipe (Opposite): | 0 | Tree: | 0 |
| Approach Turn: | 9 | Large Boulders or Rocks: | 0 |
| Overtaking Turn: | 0 | Barricade: | 0 |
| Parked Motor Vehicle: | 0 | Wall/Building: | 0 |
| Railway Vehicle: | 0 | Crash Cushion: | 0 |
| Bicycle: | 0 | Mailbox: | 0 |
| Motorized Bicycle: | 0 | Other Fixed Object: | 0 |
| Domestic Animal: | 0 | Total Fixed Objects: | 1 |
| Wild Animal: | 0 | Rocks in Roadway: | 0 |
| Light/Utility Pole: | 0 | Vehicle Cargo/Debris: | 0 |
| Traffic Signal Pole: | 0 | Road Maintenance Equipment: | 0 |
| Sign: | 0 | Involving Other Object: | 0 |
| Bridge Rail: | 0 | Total Other Objects: | 0 |
| Guard Rail: | 0 | Unknown: | 0 |
| Cable Rail: | 0 | Total: | 22 |
| Concrete Barrier: | 0 |  |  |


| -Mainline/Ramps/Frontage Roads |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mainline <br> Crossroad (A): | 0 | - Frontage/Ramp Intersections- |  |  |  |  |  |
|  | 0 | M | 0 N : | 0 | O | 0 P : | 0 |
| -Ramps |  |  |  |  |  |  |  |
| B: $\quad 0 \mathrm{~F}$ : | 0 J : | 0 | Left Frontage |  | 0 |  |  |
| C: $\quad 0 \mathrm{G}$ : | 0 K : | 0 | Rt Frontage |  | 0 |  |  |
| D: 0 H: | 0 L : | 0 | HOV Lan |  | 0 |  |  |
| E: 0 l: | 0 |  |  |  | 22 | Total: | 22 |


| Snow/Sleet/Hail: | 1 |
| ---: | ---: |
| Fog: | 0 |
| Dust: | 0 |
| Wind: | 0 |
| Unknown: | 6 |
| Total: | $\mathbf{2 2}$ |


| - Road Description |  | -Road Conditions |  |
| :---: | :---: | :---: | :---: |
| At Intersection: | 21 | Dry: | 19 |
| At Driveway Access: | 0 | Wet: | 2 |
| Intersection Related: | 1 | Muddy: | 0 |
| Non Intersection: | 0 | Snowy: | 0 |
| In Alley: | 0 | Icy: | 1 |
| Roundabout: | 0 | Slushy: | 0 |
| Ramp: | 0 | Foreign Material: | 0 |
| Parking Lot: | 0 | With Road Treatment: | 0 |
| Unknown: | 0 | Dry w/lcy Road Treatment: | 0 |
| Total: |  | Wet w/lcy Road Treatment: | 0 |
| Total. |  | Snowy w/Icy Road Treatment: | 0 |
|  |  | Icy w/Icy Road Treatment: | 0 |
|  |  | Slushy w/Icy Road Treatment: | 0 |
|  |  | Unknown: | 0 |
|  |  | Total: | 22 |




[^0]:    ${ }^{1}$ Hauer et al. Estimating Safety by the Empirical Bayes Method. In Transportation Research Record 1174, TRB, National Research Council, Washington, D.C., 2002, pp 126-131.

[^1]:    ${ }^{2}$ Hauer, E. Observational Before-After Studies in Road Safety. Pergamon, Elsevier Science Ltd, 1997.

[^2]:    ${ }^{3}$ Gross, Persaud and Lyon, Guide to Developing Quality Crash Modification Factors, Report No. FHWA-SA-10-032, December 2010.

[^3]:    ${ }^{4}$ Hauer et al. Estimating Safety by the Empirical Bayes Method. In Transportation Research Record 1174, TRB, National Research Council, Washington, D.C., 2002, pp 126-131.

